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**ATTACHMENTS DISTRIBUTED UNDER SEPARATE COVER**

**PV 19/10/21 – 42 Georgetown Road Georgetown - DA2021/00197 -  
Service station and ancillary food and drink premises including  
demolition of existing structures**

**PAGE 3    ITEM-2    Attachment A:    Submitted Plans**

**PAGE 28    ITEM -2    Attachment B:    Processing Chronology**

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**PV 19/10/21 – 42 Georgetown Road Georgetown - DA2021/00197 -  
Service station and ancillary food and drink premises including  
demolition of existing structures**

**ITEM-2          Attachment A:          Submitted Plans**

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# Proposed Development

Proposed Fast Food and Service Station

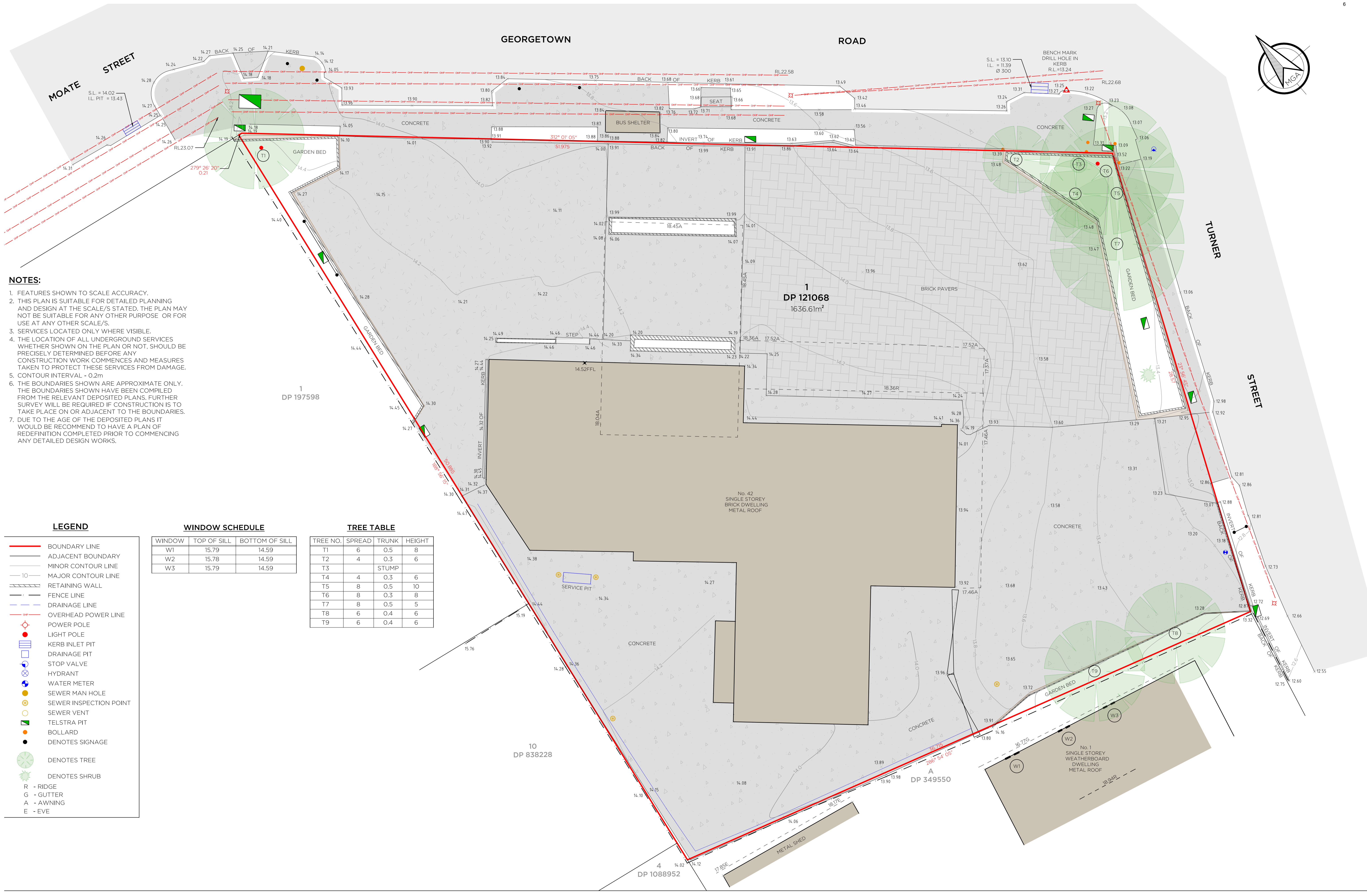
Lot 1 // DP121068; 42 Georgetown Rd, Georgetown



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- NOTES:**
1. FEATURES SHOWN TO SCALE ACCURACY.
  2. THIS PLAN IS SUITABLE FOR DETAILED PLANNING AND DESIGN AT THE SCALE/S STATED. THE PLAN MAY NOT BE SUITABLE FOR ANY OTHER PURPOSE OR FOR USE AT ANY OTHER SCALE/S.
  3. SERVICES LOCATED ONLY WHERE VISIBLE.
  4. THE LOCATION OF ALL UNDERGROUND SERVICES WHETHER SHOWN ON THE PLAN OR NOT, SHOULD BE PRECISELY DETERMINED BEFORE ANY CONSTRUCTION WORK COMMENCES AND MEASURES TAKEN TO PROTECT THESE SERVICES FROM DAMAGE.
  5. CONTOUR INTERVAL - 0.2m
  6. THE BOUNDARIES SHOWN ARE APPROXIMATE ONLY. THE BOUNDARIES SHOWN HAVE BEEN COMPILED FROM THE RELEVANT DEPOSITED PLANS. FURTHER SURVEY WILL BE REQUIRED IF CONSTRUCTION IS TO TAKE PLACE ON OR ADJACENT TO THE BOUNDARIES.
  7. DUE TO THE AGE OF THE DEPOSITED PLANS IT WOULD BE RECOMMEND TO HAVE A PLAN OF REDEFINITION COMPLETED PRIOR TO COMMENCING ANY DETAILED DESIGN WORKS.

**LEGEND**

- BOUNDARY LINE
- ADJACENT BOUNDARY
- MINOR CONTOUR LINE
- MAJOR CONTOUR LINE
- RETAINING WALL
- FENCE LINE
- DRAINAGE LINE
- OVERHEAD POWER LINE
- POWER POLE
- LIGHT POLE
- KERB INLET PIT
- DRAINAGE PIT
- STOP VALVE
- HYDRANT
- WATER METER
- SEWER MAN HOLE
- SEWER INSPECTION POINT
- SEWER VENT
- TELSTRA PIT
- BOLLARD
- DENOTES SIGNAGE
- DENOTES TREE
- DENOTES SHRUB
- R - RIDGE
- G - GUTTER
- A - AWNING
- E - EVE

**WINDOW SCHEDULE**

WINDOW	TOP OF SILL	BOTTOM OF SILL
W1	15.79	14.59
W2	15.78	14.59
W3	15.79	14.59

**TREE TABLE**

TREE NO.	SPREAD	TRUNK	HEIGHT
T1	6	0.5	8
T2	4	0.3	6
T3		STUMP	
T4	4	0.3	6
T5	8	0.5	10
T6	8	0.3	8
T7	8	0.5	5
T8	6	0.4	6
T9	6	0.4	6

EV.	DATE	AMENDMENT(S)	SUR	DFT	CHK
A	14.10.20	ORIGINAL ISSUE	RT	PV	TR

**DELFS LASCELLES**  
CONSULTING SURVEYORS  
260 MAITLAND ROAD, MAYFIELD NSW 2304  
T: (02) 4964 4886  
E: admin@delacs.com.au

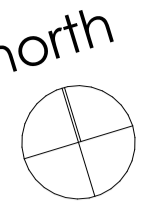
**DETAIL SURVEY OF LOT 1 DP 121068**

SITE ADDRESS:  
42 GEORGETOWN ROAD  
GEORGETOWN  
CLIENT:  
GWH BUILD

CAD REF: 20596 - DET

POSITION DATUM: PM 14519  
ORIENTATION: MGA (GROUND)  
EASTING: 381 330.962  
NORTHING: 6 358 056.047  
CLASS: B  
HEIGHT DATUM: PM 14519  
CLASS: LB

SURVEYED	DRAFTED	CHECKED
RT	PV	TR
SCALE	PAGE SIZE	DATE
1:100	A1	07.10.20
SHEET	REV.	PROJECT NO.
1	A	20596



**General Note**

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All boundaries and contours are subject to survey. All levels to Australian Height Data. It is the contractors responsibility to confirm all measurements on site and locations of any services prior to work on site.

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**Site**

Site area (approx.) 1 636.61m<sup>2</sup>

External concrete area 1 192.26m<sup>2</sup>

**Carparking**

**Service Station**  
Newcastle DPC 2012 - 7.03 Table 1  
1 space per 20m<sup>2</sup> GFA of convenience store

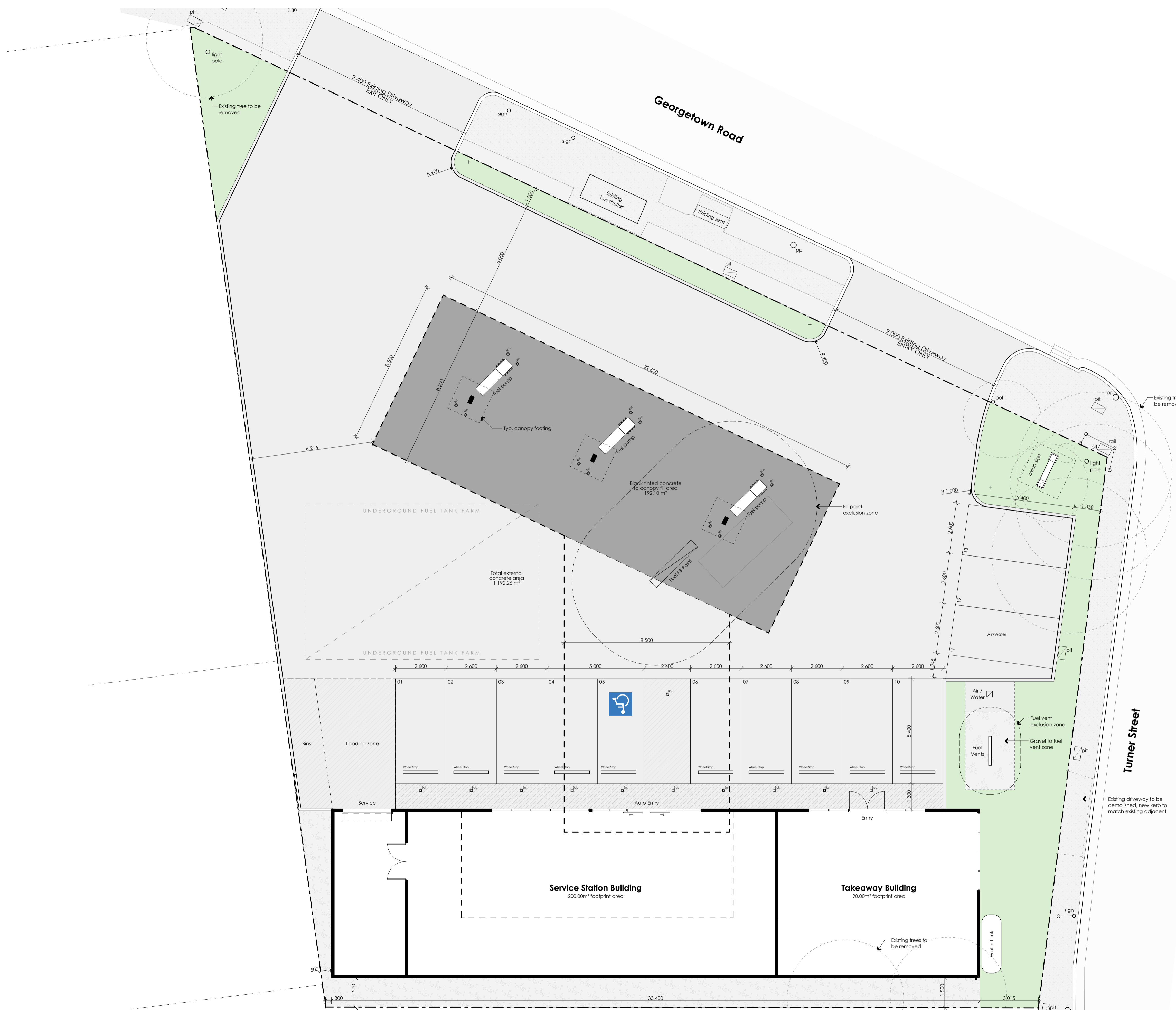
Additional parking required if development includes restaurant or takeaway food outlet.

77m<sup>2</sup> / 20m<sup>2</sup> = 3.85 Spaces

**Fast Food**  
Newcastle DPC 2012 - 7.03 Table 1  
Developments with no on-site seating  
12 spaces per 100m<sup>2</sup> GFA

90.00m<sup>2</sup> / 12 per 100m<sup>2</sup> = 10.8 Spaces

15 Spaces required  
13 Spaces provided



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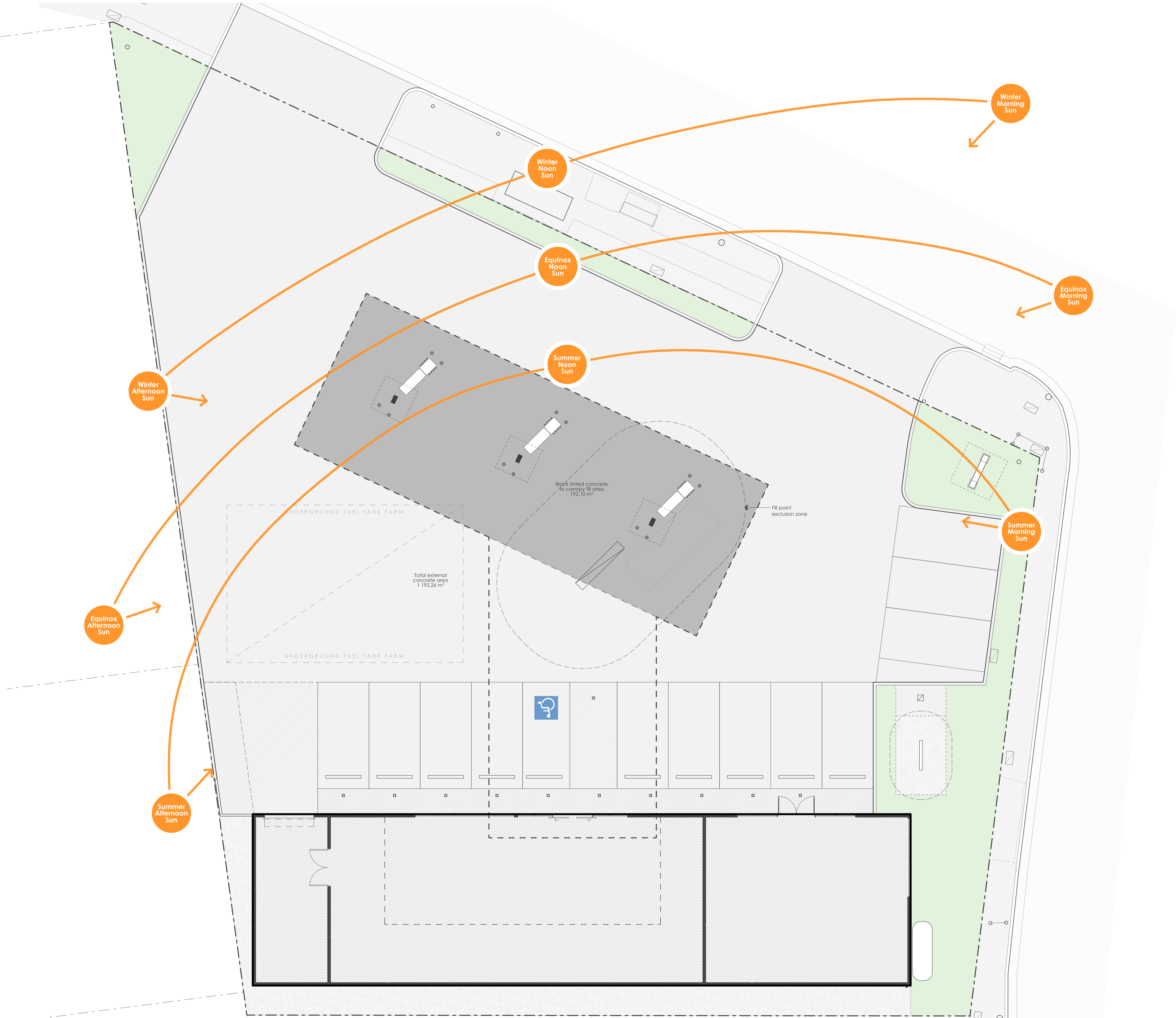
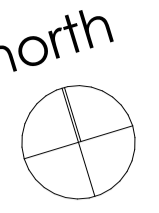
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01 - WIP  
rev no. Prelim DA  
description **Preliminary**  
Work in Progress  
date

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Suite 1  
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Newcastle, NSW 2303

Proposed Development  
Lot 1 // DP121068  
42 Georgetown Rd  
Georgetown, NSW 2298

Proposed Site Plan  
project no. A19028  
date: 2021/01/13  
drawn: BT / KB  
dwg no. A002  
scale: 1:100 @ A1  
rev no. 01 - WIP



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description

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Proposed Development

Lot 1 // DP121068  
42 Georgetown Rd  
Georgetown, NSW 2298

Site Analysis

project no. A19028  
date: 2021/01/13  
drawn: BT /KB

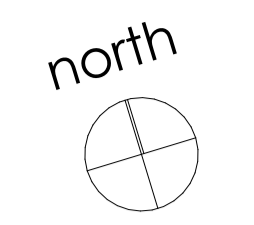
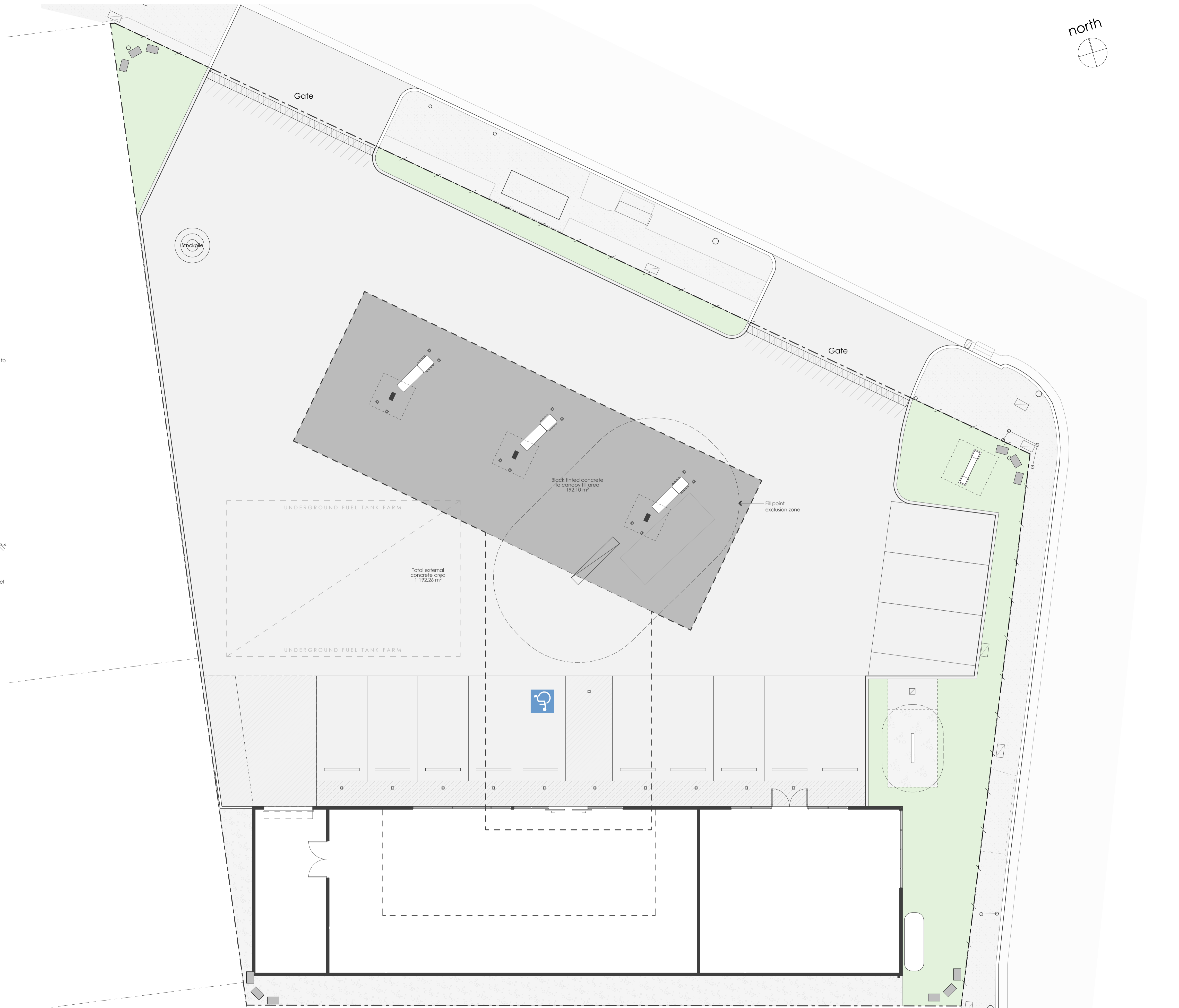
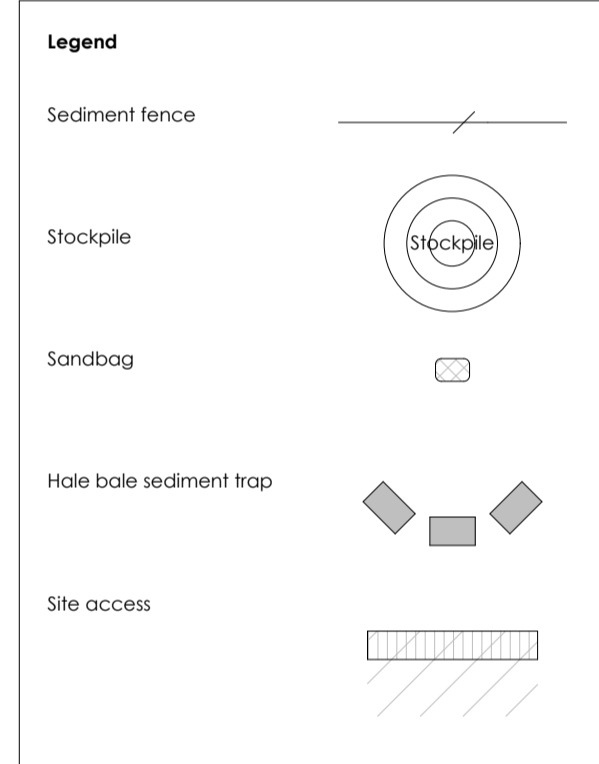
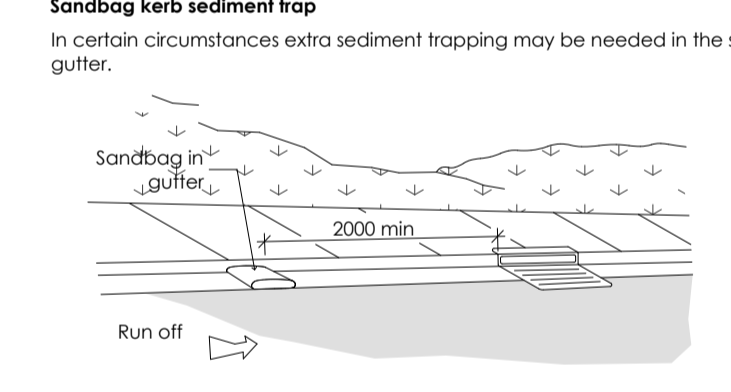
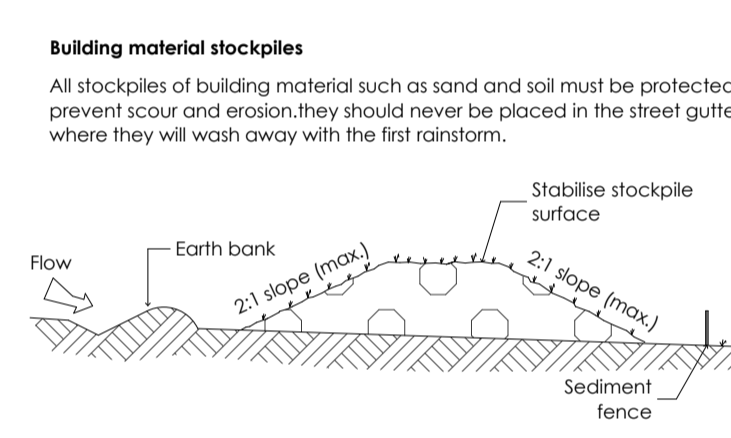
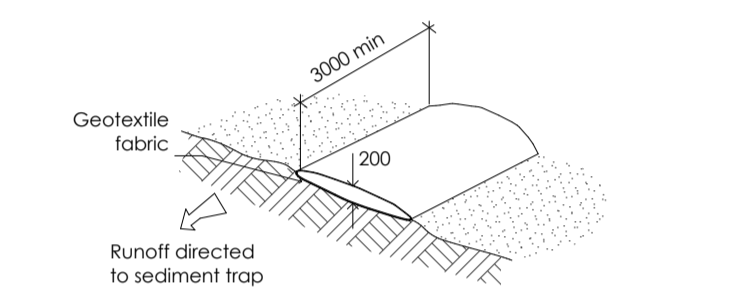
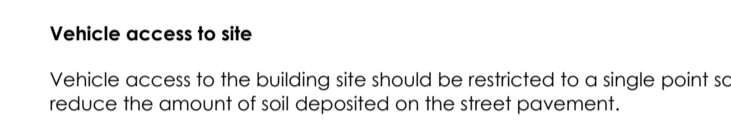
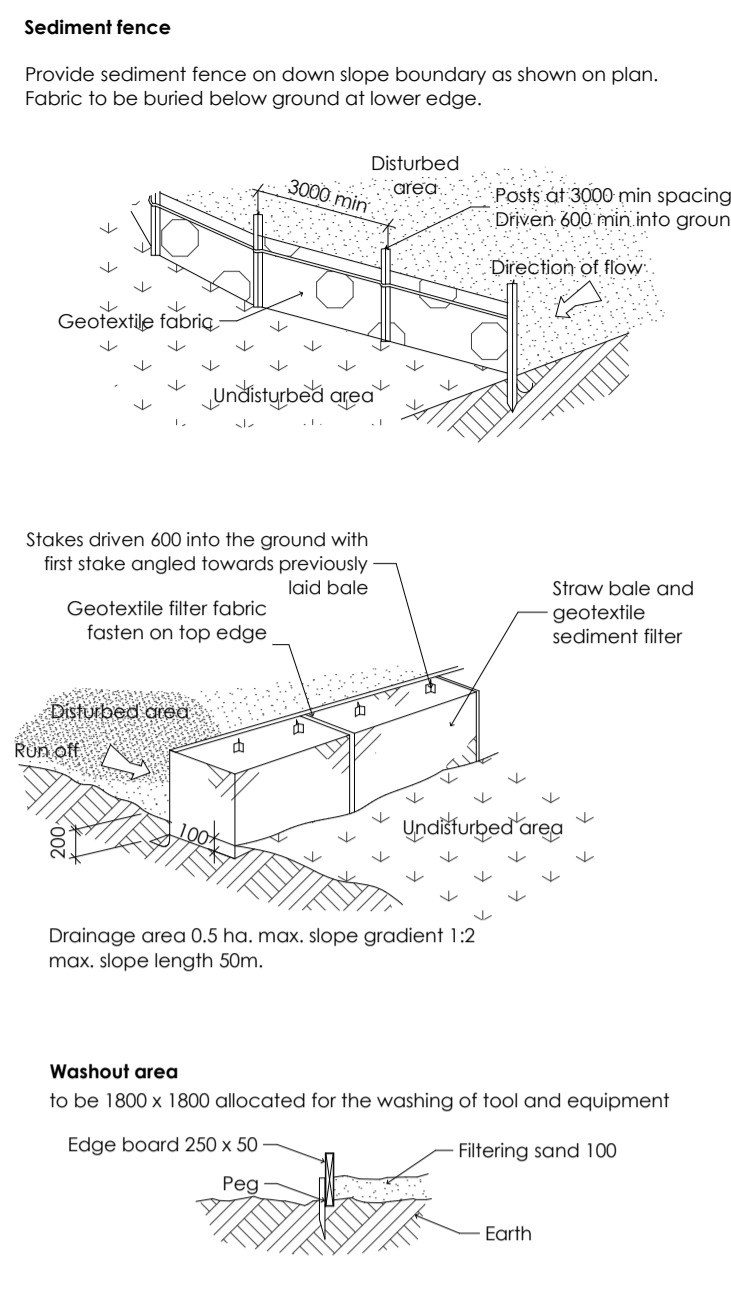
dwg no. A003  
scale: 1:100, 1:1 @ A1  
rev no. 01 - WIP



**Sediment and erosion control**  
 All erosion and sediment control measures to be installed prior to site disturbance.  
 The project manager to inform all contractors and sub-contractors of their obligations under the erosion and sediment control plan.  
 Topsoil from all areas that will be disturbed to be stripped and stockpiled at the nominated site.  
 All sediment control structures to be inspected by site supervisor after each rainfall event for structural damage and all trapped sediment to be removed to a nominated stockpile site.  
 Sediment and erosion control shall be effectively maintained at all times during the course of construction and shall not be removed until the site has been stabilised or landscaped to the superintendent's satisfaction.  
 A single all weather access way will be provided at the front of the property consisting of 50-75 aggregate or similar material of a minimum thickness of 150 laid over needle-punched geotextile fabric and constructed prior to commencement of works.  
 The contractor shall ensure that no spoil or fill encroaches upon adjacent areas for the duration of works.  
 The contractor shall ensure that kerb inlets and drains receiving stormwater shall be protected at all times during development. Kerb inlet sediment traps shall be installed along the immediate vicinity along the street frontage.  
 Sediment fencing shall be secured by post (where metal star pickets are used plastic safety caps shall be used) of 3000 intervals with geotextile fabric embedded 200 in soil.  
 All topsoil stripped from the site and stockpiled does not interfere with drainage lines and stormwater inlets and will be suitably covered with an impervious membrane material and screened by sediment fencing.

**Soil conservation**  
 Prior to commencement of construction provide 'sediment fence', 'sediment trap' and washout area to ensure the capture of water borne material generated from the site.  
 Maintain the above during the course of construction, and clear the 'sediment trap' after each storm.

**Sediment trap**  
 1000 x 1000 wide 500 deep pit, located at the lowest point to the trap sediment.



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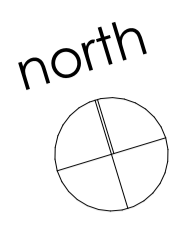
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 description  
 Work in Progress  
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Proposed Development  
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Sediment Control Plan  
 project no. A19028  
 date: 2021/01/13  
 drawn: BT /KB  
 dwg no. A004  
 scale: 1:100 @ A1  
 rev no. 01 - WIP

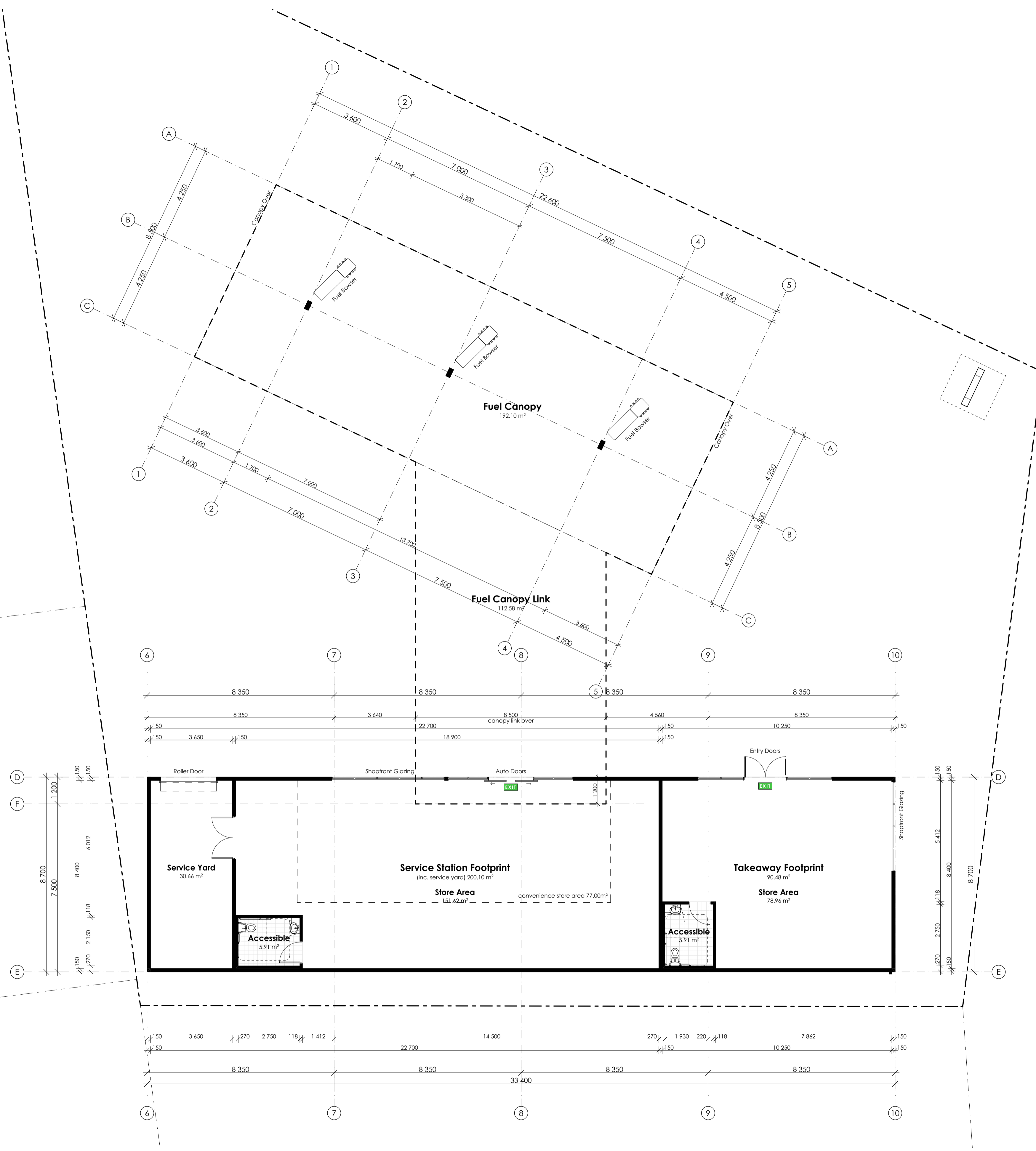


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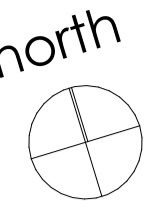
**Proposed Development**

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Georgetown, NSW 2298

**Ground Floor**

project no. A19028  
date: 2021/01/13  
drawn: BT /KB

dwg no. A101  
scale: 1:100 @ A1  
rev no. 01 - WIP

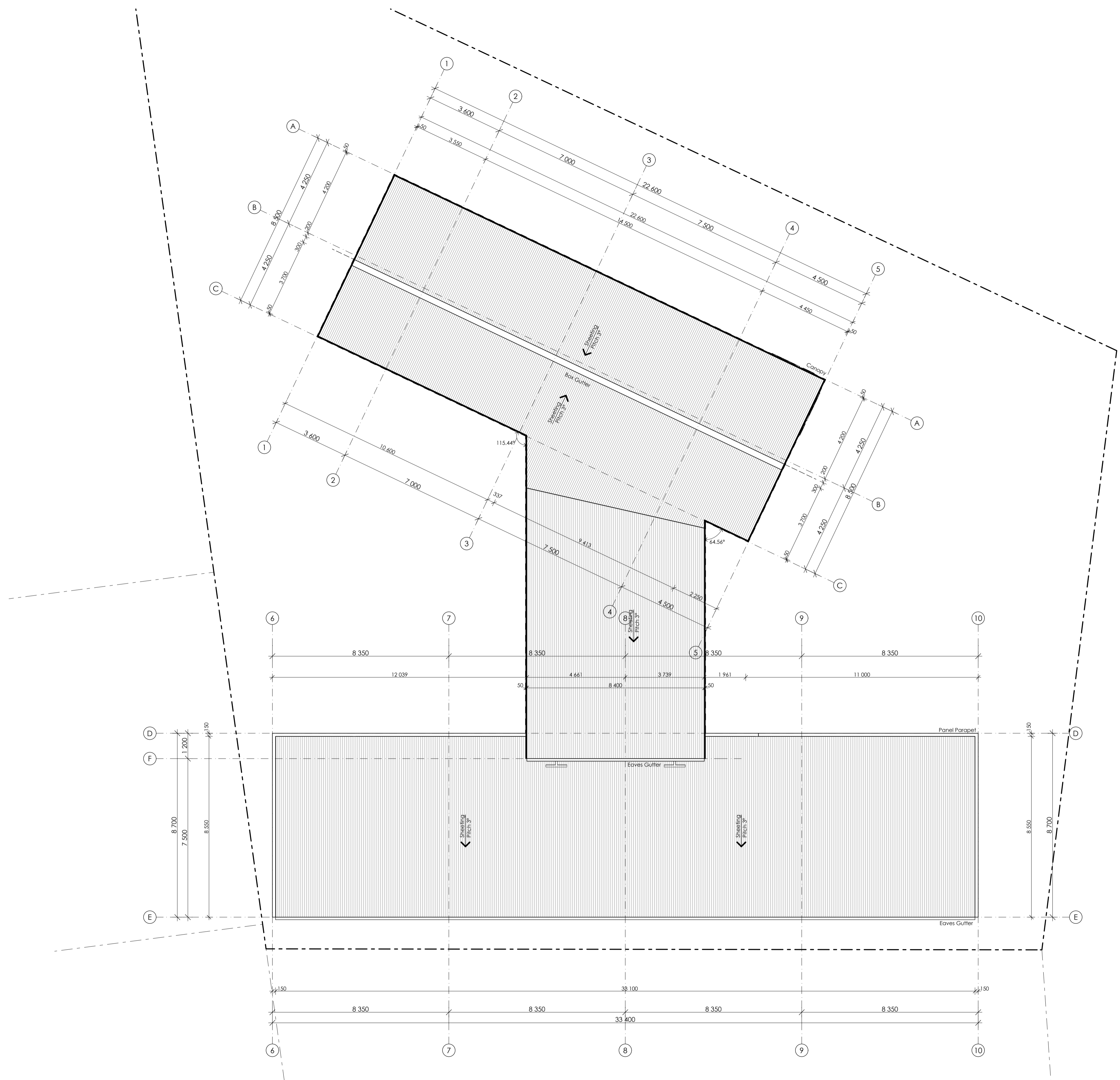


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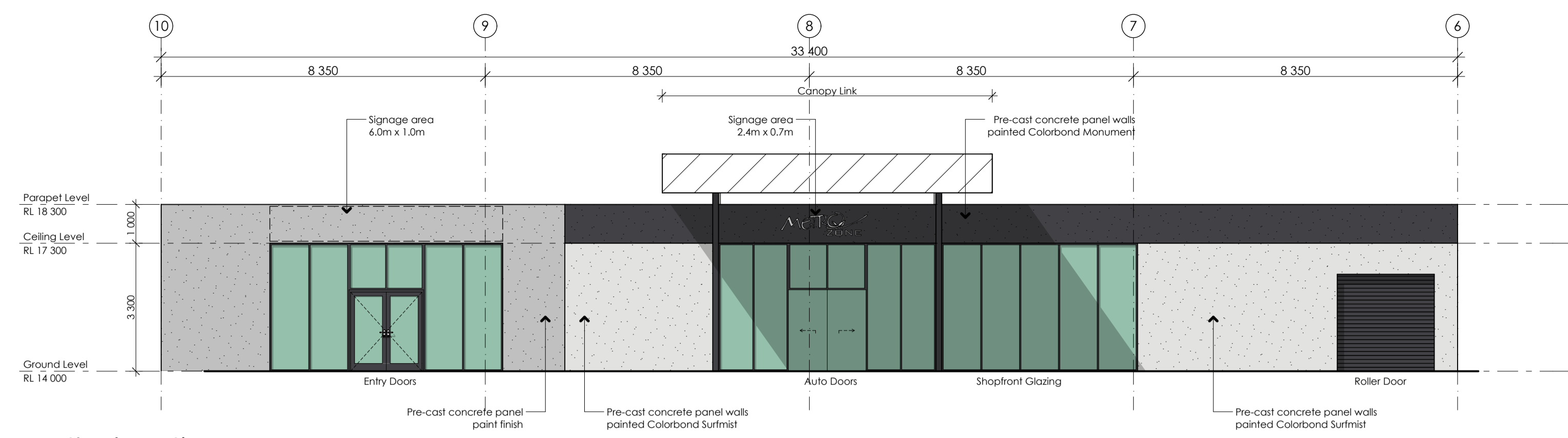
**Proposed Development**

Lot 1 // DP121068  
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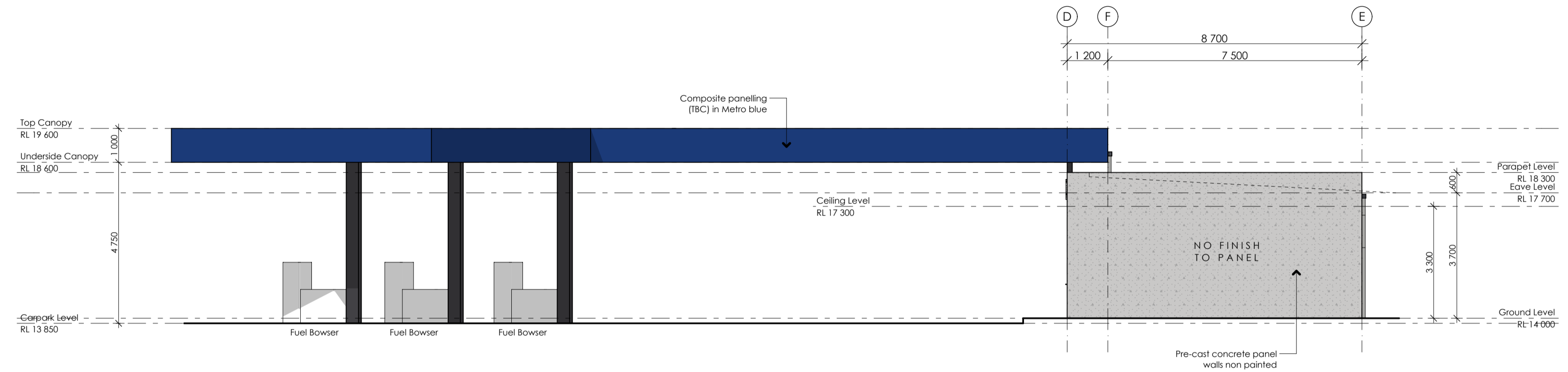
**Roof Plan**

project no. A19028  
date: 2021/01/13  
drawn: BT /KB

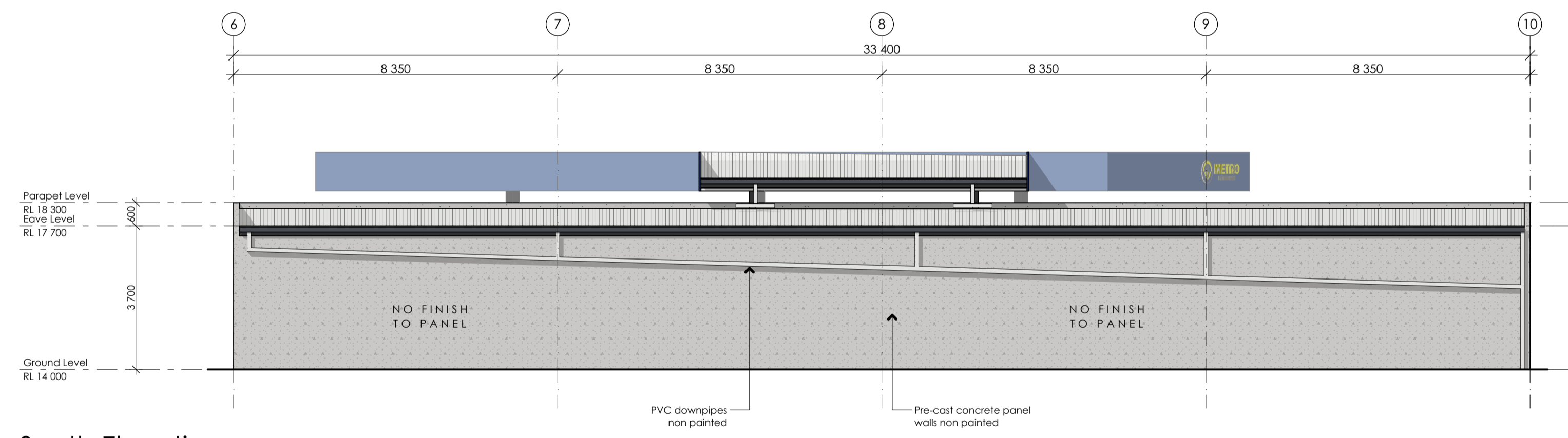
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rev no. 01 - WIP



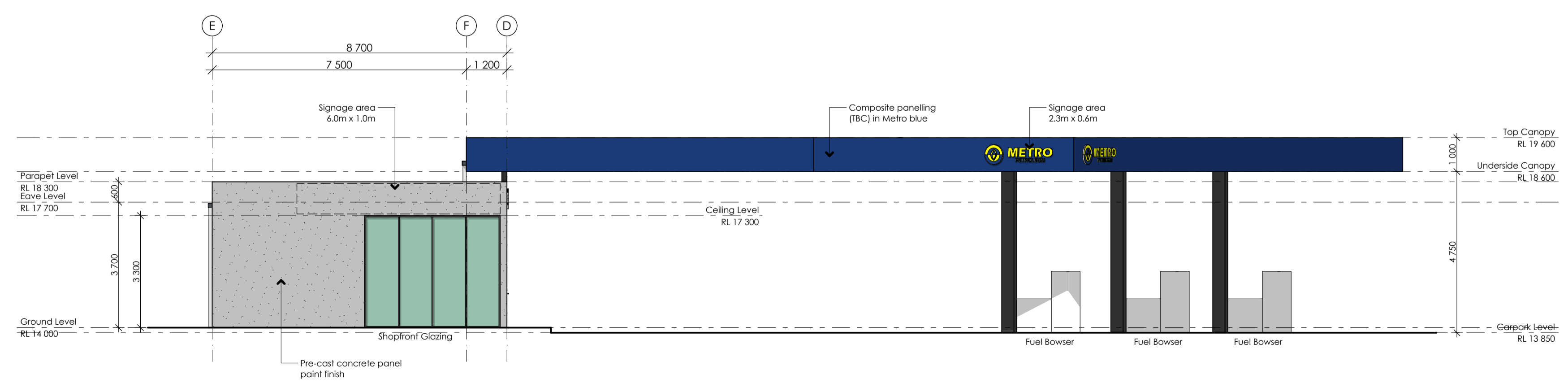
North Elevation  
1:100



West Elevation  
1:100



South Elevation  
1:100



East Elevation  
1:100



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description date  
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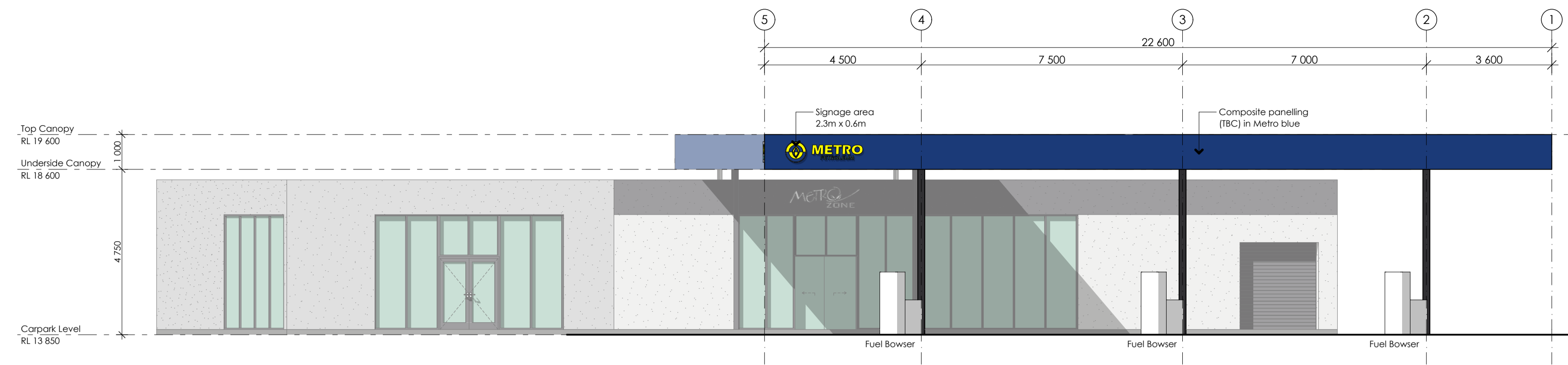
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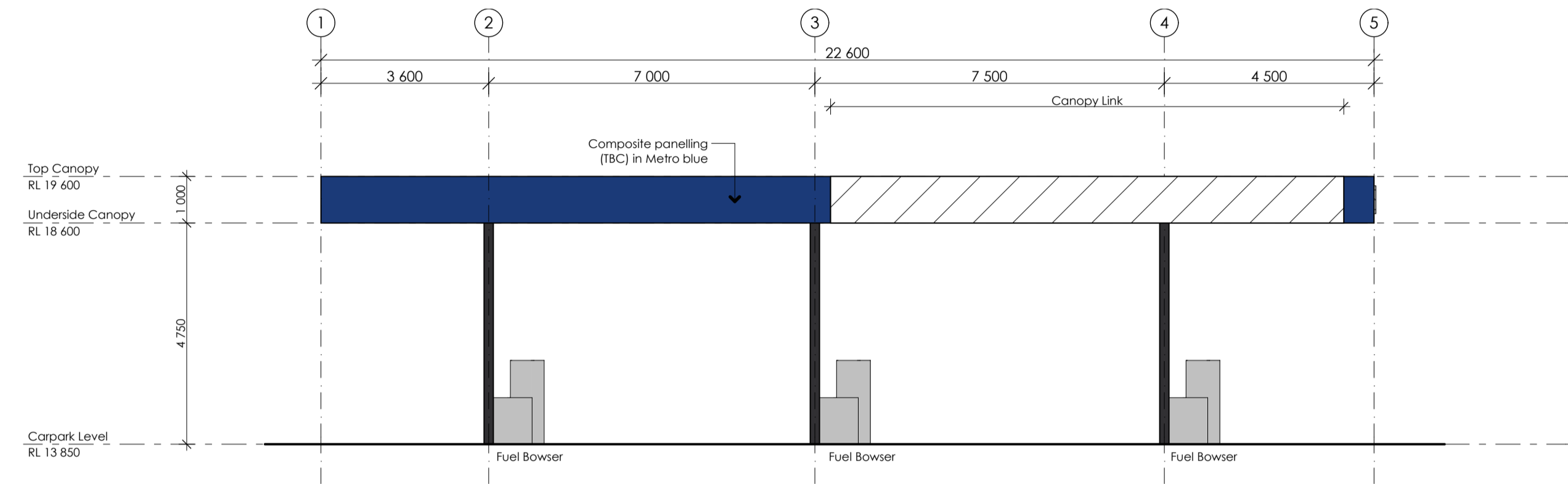
Elevations

project no. A19028  
date: 2021/01/13  
drawn: BT /KB

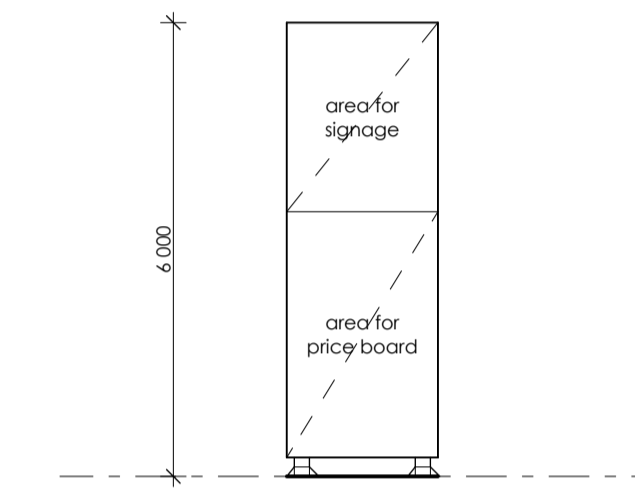
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scale: 1:100 @ A1  
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Canopy - North Elevation  
1:100



Canopy - South Elevation  
1:100



Pylon Signage  
1:100



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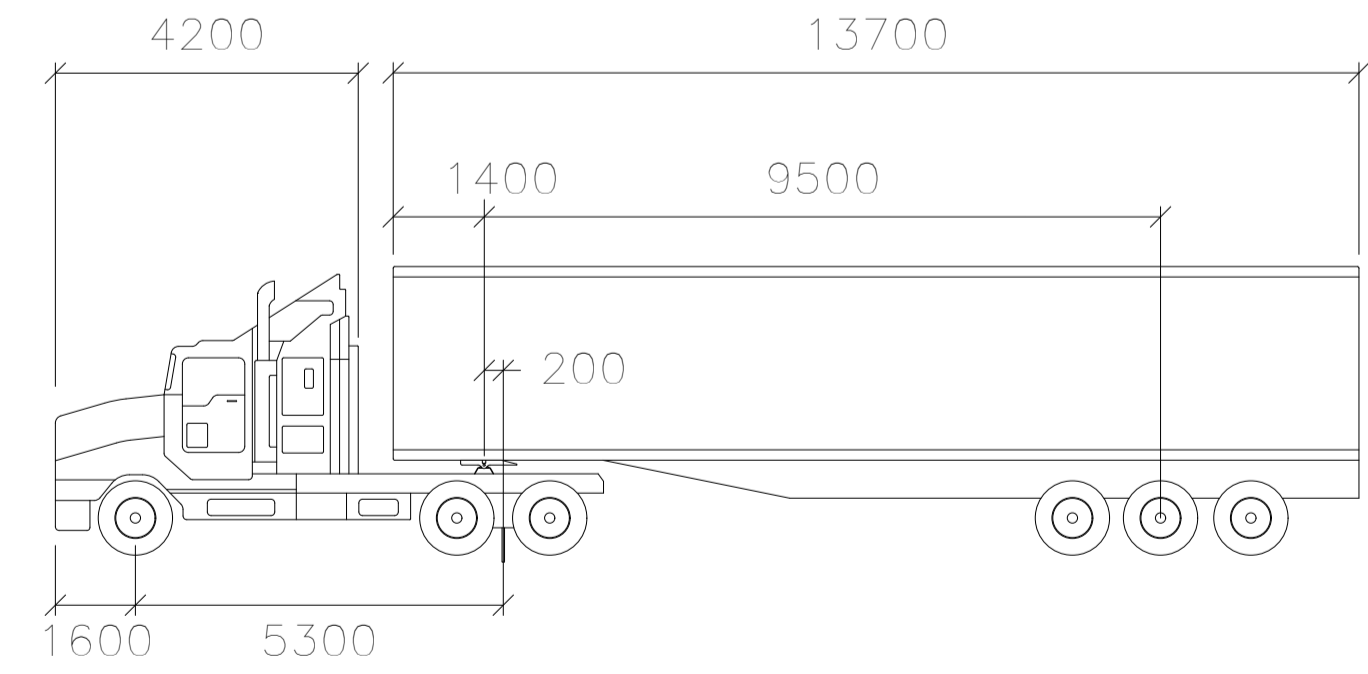
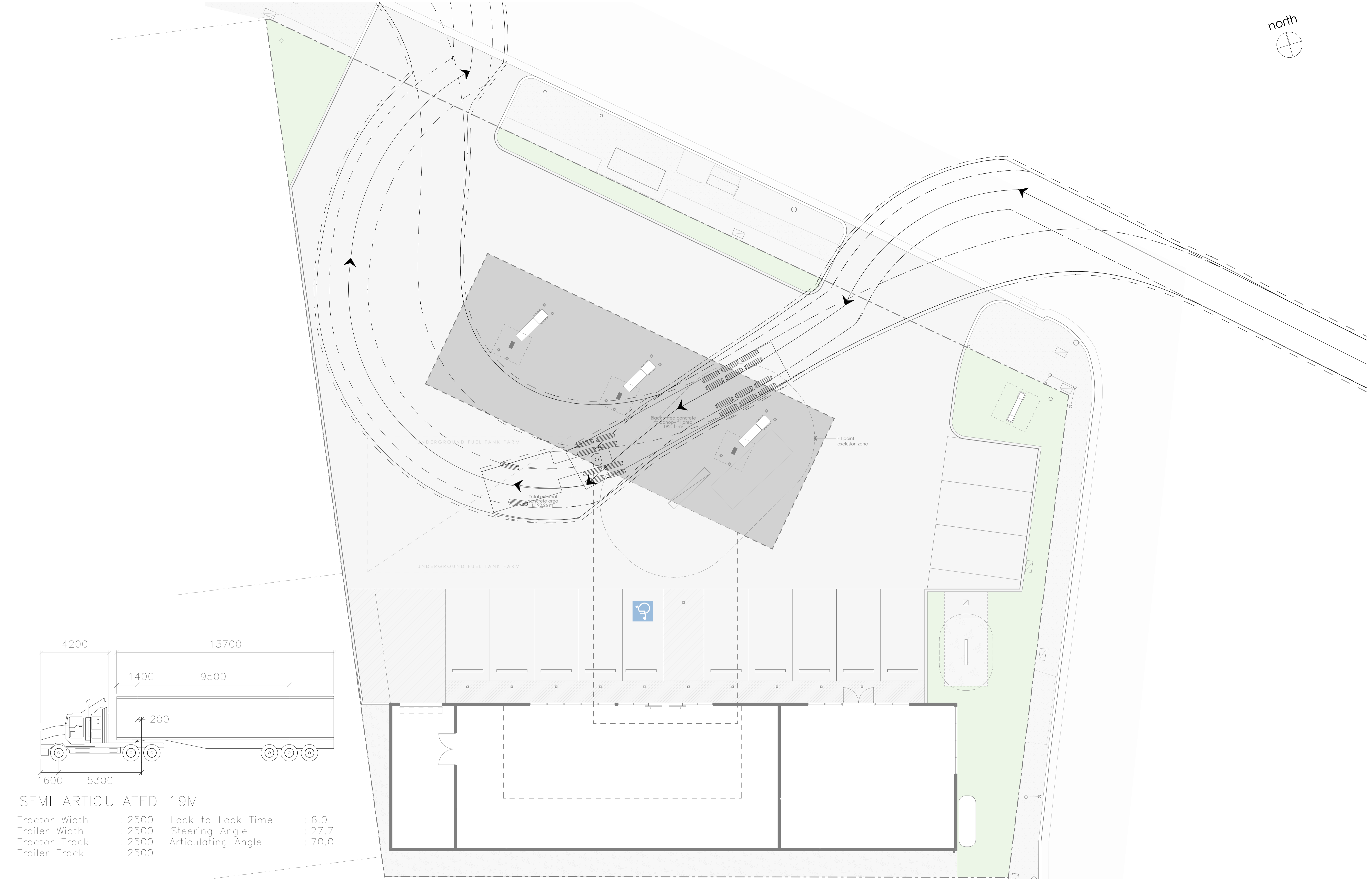
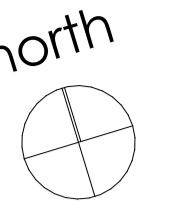
Proposed Development

Lot 1 // DP121068  
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Elevations

project no. A19028  
date: 2021/01/13  
drawn: BT /KB

dwg no. A502  
scale: 1:100 @ A1  
rev no. 01 - WIP



SEMI ARTICULATED 19M

Tractor Width	: 2500	Lock to Lock Time	: 6.0
Trailer Width	: 2500	Steering Angle	: 27.7
Tractor Track	: 2500	Articulating Angle	: 70.0
Trailer Track	: 2500		



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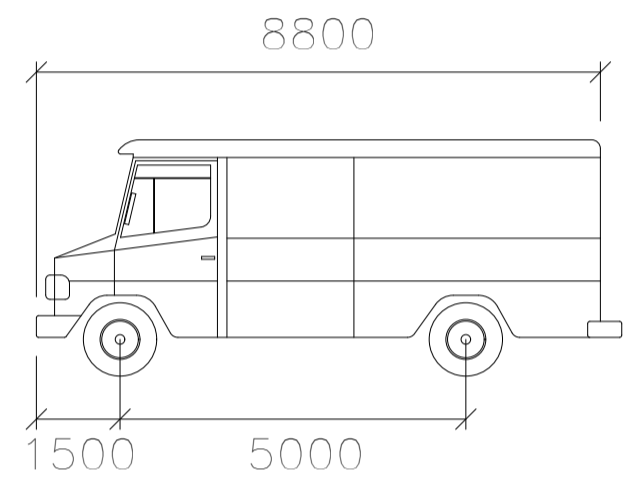
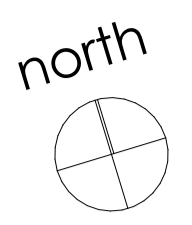
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Proposed Development  
Lot 1 // DP121068  
42 Georgetown Rd  
Georgetown, NSW 2298

Turning Template | 19m B-Double  
project no. A19028  
date: 2021/01/13  
drawn: BT /KB  
dwg no. A1002  
scale: 1:100 @ A1  
rev no. 01 - WIP



**SERVICE VEHICLE**  
 Width : 2500  
 Track : 2500  
 Lock to Lock Time : 6.0  
 Steering Angle : 38.7



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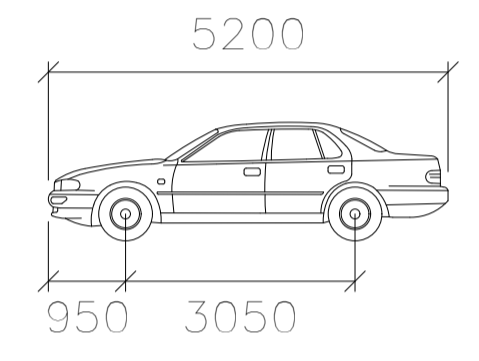
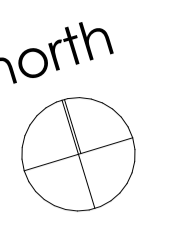
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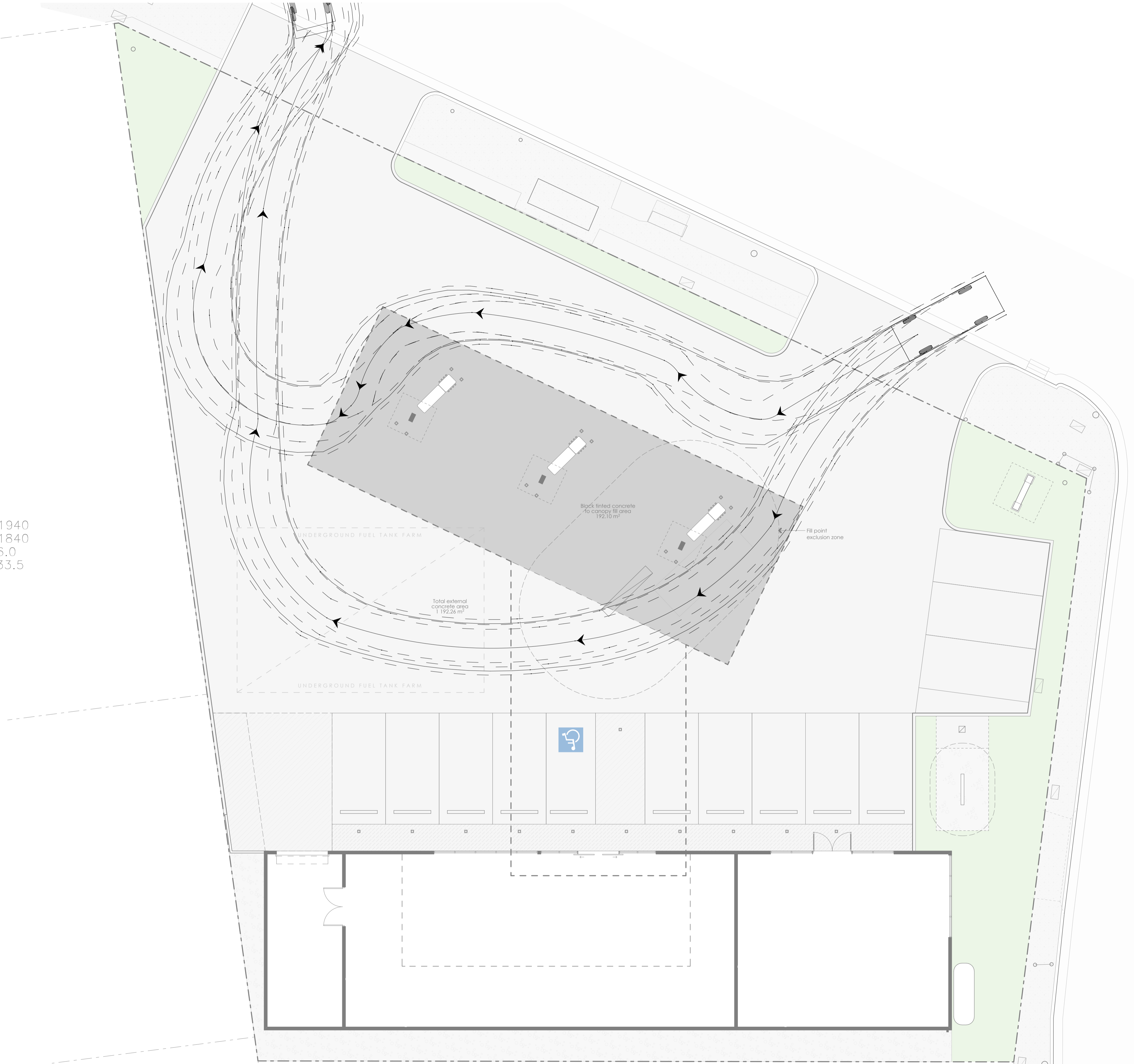
**Proposed Development**  
 Lot 1 // DP121068  
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**Turning Template | Service Vehicle**  
 project no. A19028  
 date: 2021/01/13  
 drawn: BT /KB  
 dwg no. A1003  
 scale: 1:100 @ A1  
 rev no. 01 - WIP



**B85 CAR**

- Width : 1940
- Track : 1840
- Lock to Lock Time : 6.0
- Steering Angle : 33.5



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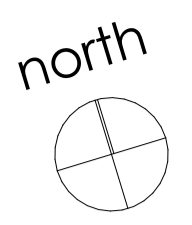
01 - WIP  
rev no. **Preliminary**  
description  
date Work in Progress

**GWH**  
Suite 1  
770 Hunter Street  
Newcastle, NSW 2303

**Proposed Development**  
Lot 1 // DP121068  
42 Georgetown Rd  
Georgetown, NSW 2298

**Turning Template | B85 Car**  
project no. A19028  
date: 2021/01/13  
drawn: BT /KB  
dwg no. A1004  
scale: 1:100 @ A1  
rev no. 01 - WIP



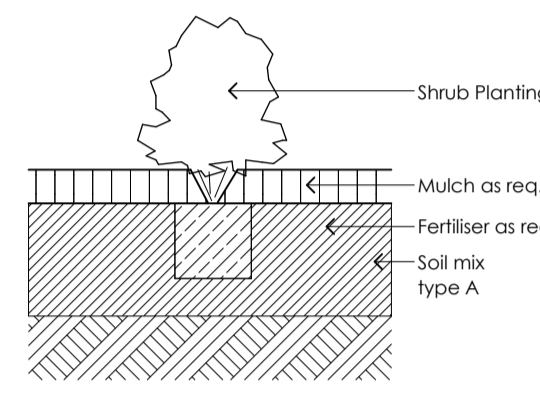
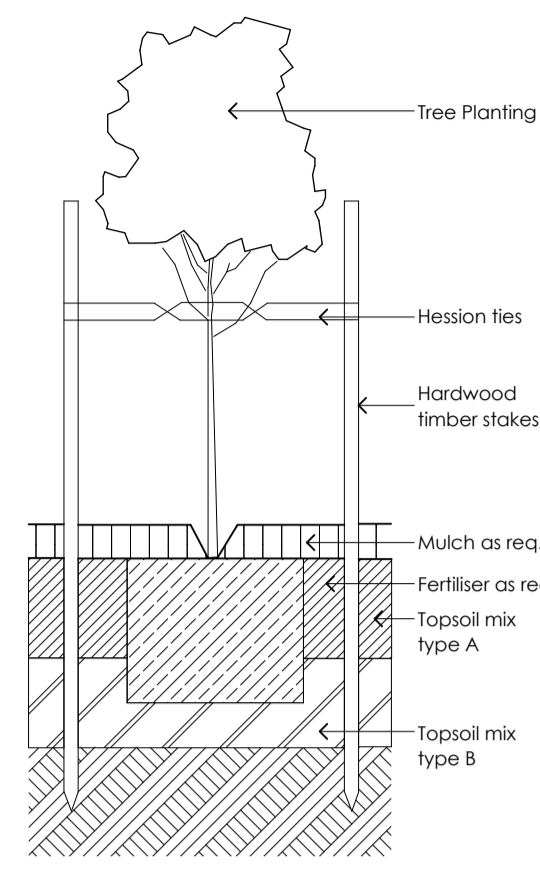


**General Note**

The Builder shall check all dimensions and levels on site prior to construction. Refer to written dimensions only. Do not scale drawings. Drawings shall not be used for construction purposes until issued for construction and read in conjunction with Specification and Engineers Documentation.

All boundaries and contours are subject to survey. All levels to Australian Height Data. It is the contractors responsibility to confirm all measurements on site and locations of any services prior to work on site.

All workmanship and materials to be in accordance with relevant Australian Standards, Codes and Authority. All materials to be installed to manufacturers specifications



- Accent Planting**  
Raphiolepis delacourii x indica 'Ballarina'  
(Indian Hawthorn)
- Shrub**  
Syzygium australe 'Express'  
(Lily Pilly)
- Tree**  
Tristaniopsis laurina 'Luscious'  
(Water Gum)



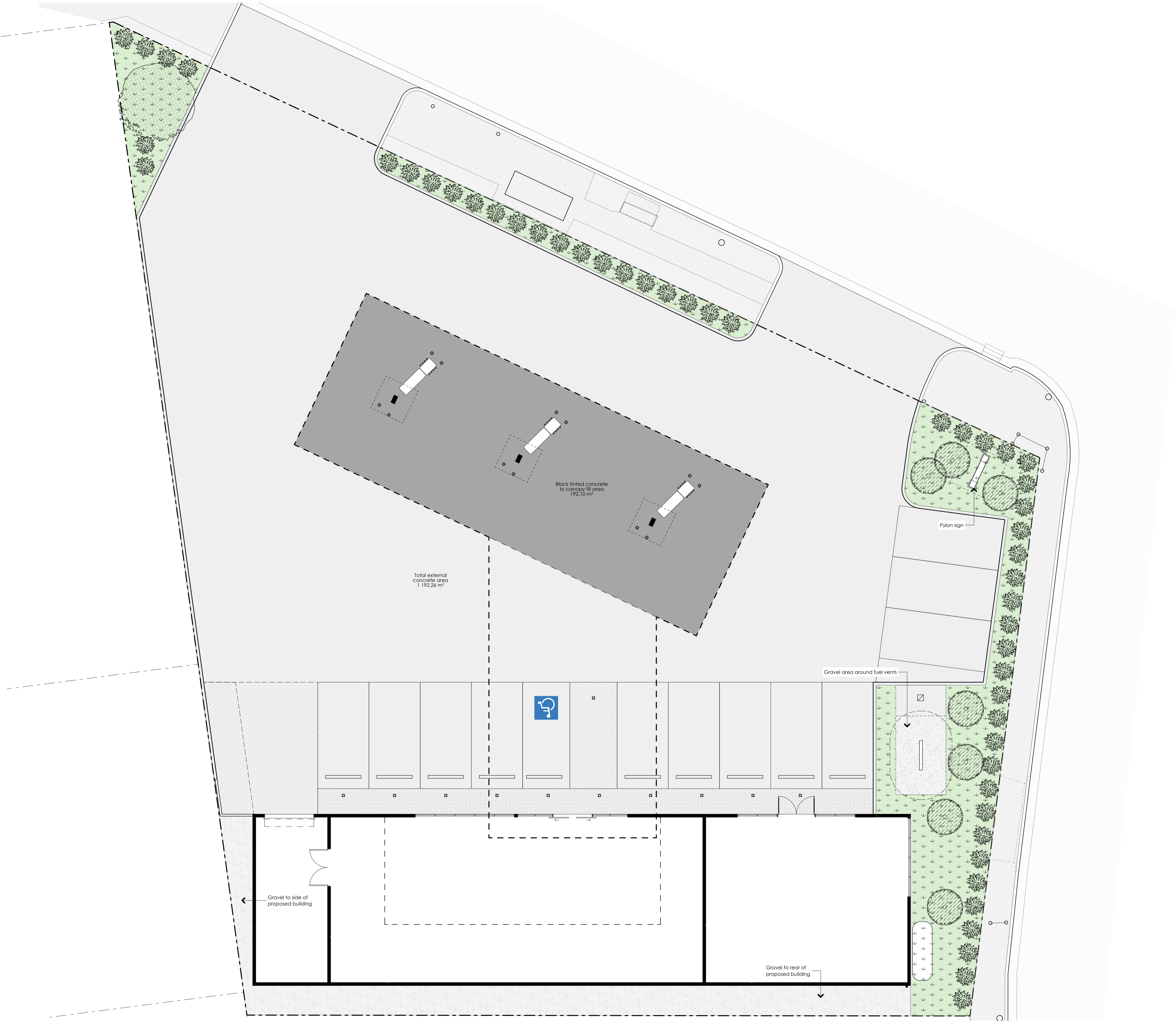
**Accent Planting**  
Raphiolepis delacourii x indica 'Ballarina'  
(Indian Hawthorn)



**Shrub**  
Syzygium australe 'Express'  
(Lily Pilly)



**Tree**  
Tristaniopsis laurina 'Luscious'  
(Water Gum)



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01 - WIP	Prelim DA	Work in Progress
rev no.	description	date

**Preliminary**

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770 Hunter Street  
Newcastle, NSW 2303

Proposed Development  
Lot 1 // DP121068  
42 Georgetown Rd  
Georgetown, NSW 2298

project no.	A19028	dwg no.	A1001
date:	2021/01/13	scale:	1:100, 1:200 @ A1
drawn:	BT /KB	rev no.	01 - WIP

Proposed Landscape Plan

# PROPOSED DEVELOPMENT

## 42 Georgetown Road, Georgetown

### CONCEPT STORMWATER / CIVIL WORKS



#### GENERAL NOTES

- G1. THE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL DRAWINGS AND SPECIFICATIONS AND OTHER WRITTEN INSTRUCTIONS THAT MAY BE ISSUED.
- G2. DIMENSIONS SHALL NOT BE OBTAINED BY SCALING FROM THE DRAWINGS. REFER ARCHITECTS DRAWINGS FOR ALL DIMENSIONS.
- G3. REFER ANY DISCREPANCY TO THE ENGINEER/ARCHITECT.
- G4. MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE APPROPRIATE SAA SPECIFICATIONS OR CODE AND WITH THE REQUIREMENTS OF THE RELEVANT LOCAL AUTHORITY.
- G5. THE ALIGNMENT AND LEVEL OF ALL SERVICES SHOWN ARE APPROXIMATE ONLY. THE CONTRACTOR SHALL CONFIRM THE POSITION AND LEVEL OF ALL SERVICES PRIOR TO COMMENCEMENT OF CONSTRUCTION. ANY DAMAGE TO SERVICES SHALL BE RECTIFIED AT THE CONTRACTORS EXPENSE.
- G6. NO WORKS ARE TO COMMENCE UNTIL THE REQUIRED TREE REMOVAL PERMITS HAVE BEEN GRANTED BY RELEVANT LOCAL AUTHORITY, AND THE APPROPRIATE NOTICE OF INTENTION TO COMMENCE GIVEN.
- G7. ALL SERVICES, OR CONDUITS FOR SERVICING SHALL BE INSTALLED PRIOR TO COMMENCEMENT OF PAVEMENT CONSTRUCTION.
- G8. SUBSOIL DRAINAGE, COMPRISING 100 AGRICULTURE PIPE IN GEO-STOCKING TO BE PLACED AS SHOWN AND AS MAY BE DIRECTED BY THE SUPERINTENDENT. SUBSOIL DRAINAGE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE RELEVANT LOCAL AUTHORITY CONSTRUCTION SPECIFICATION.
- G9. NO WORK IS PERMITTED WITHIN ADJOINING PROPERTIES WITHOUT WRITTEN PERMISSION FROM THE OWNERS OR RESPONSIBLE AUTHORITY.

#### DRAINAGE NOTES

- D1. ALL DRAINAGE OUTLET LEVELS SHALL BE CONFIRMED ON SITE, PRIOR TO CONSTRUCTION COMMENCING.
- D2. ALL PIPES WITHIN THE PROPERTY TO BE MIN. 100 DIA UPVC @ 1% MIN. GRADE. UNO.
- D3. ALL PITS WITHIN THE PROPERTY ARE TO BE FITTED WITH "WELDLOK" OR APPROVED EQUIVALENT GRATES.
  - LIGHT DUTY FOR LANDSCAPED AREAS
  - HEAVY DUTY WHERE SUBJECTED TO VEHICULAR TRAFFIC
- D4. PITS WITHIN THE PROPERTY MAY BE CONSTRUCTED AS:
  - 1) PRECAST STORMWATER PITS
  - 2) CAST INSITU MASS CONCRETE
  - 3) CEMENT RENDERED 230mm BRICKWORK
 SUBJECT TO THE RELEVANT LOCAL AUTHORITY CONSTRUCTION SPECIFICATION.
- D5. ENSURE ALL GRATES TO PITS ARE SET BELOW FINISHED SURFACE LEVEL WITHIN THE PROPERTY. TOP OF PIT RLS ARE APPROXIMATE ONLY AND MAY BE VARIED SUBJECT TO APPROVAL OF THE ENGINEER. ALL INVERT LEVELS ARE TO BE ACHIEVED.
- D6. ANY PIPES BENEATH RELEVANT LOCAL AUTHORITY ROAD TO BE RUBBER RING JOINTED RCP. UNO.
- D7. ALL PITS IN ROADWAYS ARE TO BE FITTED WITH HEAVY DUTY GRATES WITH LOCKING BOLTS AND CONTINUOUS HINGE.
- D8. PROVIDE STEP IRONS TO STORMWATER PITS GREATER THAN 1200 IN DEPTH.
- D9. TRENCH BACK FILL IN ROADWAYS SHALL COMPRISE SHARP, CLEAN GRANULAR BACK FILL IN ACCORDANCE WITH THE RELEVANT LOCAL AUTHORITY SPECIFICATION TO NON-TRAFFICABLE AREAS TO BE COMPACTED BY RODDING AND TAMPING USING A FLAT PLATE VIBRATOR.
- D10. WHERE A HIGH EARLY DISCHARGE (HED) PIT IS PROVIDED ALL PIPES ARE TO BE CONNECTED TO THE HED PIT. UNO.
- D11. DOWN PIPES SHALL BE A MINIMUM OF DN100 SW GRADE UPVC OR 100X100 COLORBOND/ZINCALUME STEEL. UNO.
- D12. COLORBOND OR ZINCALUME STEEL BOX GUTTERS SHALL BE A MINIMUM OF 450 WIDE X 150 DEEP.
- D13. EAVES GUTTERS SHALL BE A MINIMUM OF 125 WIDE X 100 DEEP (OR OF EQUIVALENT AREA) COLORBOND OR ZINCALUME STEEL. UNO.
- D14. SUBSOIL DRAINAGE SHALL BE PROVIDED TO ALL RETAINING WALLS & EMBANKMENTS, WITH THE LINES FEEDING INTO THE STORMWATER DRAINAGE SYSTEM. UNO.

#### EARTHWORKS NOTES

- E1. THE EARTHWORKS SHALL BE CARRIED OUT IN ACCORDANCE WITH THE PROJECT GEOTECHNICAL REPORT.
- E2. THE SITE OF THE WORKS SHALL BE PREPARED BY STRIPPING ALL EXISTING TOPSOIL, FILL AND VEGETATION.
- E3. SUBGRADE SHALL BE COMPACTED UNTIL A DRY DENSITY HAS BEEN ACHIEVED OF NOT LESS THAN 100% OF THE STANDARD MAXIMUM DRY DENSITY WHEN TESTED IN ACCORDANCE WITH AS 1289 TESTS E.1.1. OR E.1.2.
- E4. THE EXPOSED SUBGRADE SHOULD BE PROOF ROLLED TO DETECT ANY SOFT OR WET AREAS WHICH SHOULD BE LOCALLY EXCAVATED AND BACK FILLED WITH SELECTED MATERIAL.
- E5. THE BACK FILLING MATERIAL SHALL BE IMPORTED GRANULAR FILL OF LOW PLASTICITY, PREFERABLY CRUSHED SANDSTONE, AND TO BE PLACED IN LAYERS NOT EXCEEDING 150 LOOSE THICKNESS AND COMPACTED TO 98% OF STANDARD DRY DENSITY AT A MOISTURE CONTENT WITHIN 2% OF OPTIMUM.
- E6. SITE WORKS ARE TO BE BATTERED TO ADJACENT PROPERTY LEVELS.
- E7. STORMWATER MUST NOT BE CONCENTRATED ON TO AN ADJACENT PROPERTY.
- E8. AT NO TIME DURING OR AFTER CONSTRUCTION IS STORMWATER TO BE PONDED ON ADJOINING PROPERTIES.
- E9. THE SITE SHALL BE GRADED AND DRAINED SO THAT STORMWATER WILL BE DIRECTED AWAY FROM THE BUILDING PLATFORM.
- E10. STORMWATER DRAINAGE SHALL BE PROVIDED AND MAINTAINED THROUGHOUT THE COURSE OF CONSTRUCTION. ALL STORMWATER RUNOFF SHALL BE GRADED AWAY FROM THE SITE WORKS AND DISPOSED OF VIA SURFACE CATCHDRAINS AND STORMWATER COLLECTION PITS.
- E11. ALL SURFACE CATCH DRAINS SHALL BE GRADED AT 1% (1 IN 100) MINIMUM. THE GROUND SHALL GRADE AWAY FROM ANY DWELLING AT 5% (1 IN 20) FOR THE FIRST METRE THEN AT 2.5% (1 IN 40).
- E12. WHERE A CUT FILL PLATFORM IS USED THERE SHALL BE A MINIMUM BERM 1000 WIDE TO THE PERIMETER OF THE SITE WORKS WHICH SHALL BE SUPPORTED BY BATTERS OF 3:1 IN FILL.
- E13. ANY VERTICAL OR NEAR VERTICAL PERMANENT EXCAVATION (CUT) DEEPER THAN 600 IN MATERIAL OTHER THAN ROCK SHALL BE ADEQUATELY RETAINED OR BATTERED AT A MINIMUM OF 3:1.
- E14. WHERE BATTERS CANNOT BE PROVIDED TO SUPPORT THE CUT OR FILL, THEY SHALL BE ADEQUATELY RETAINED.
- E15. RETAINING WALLS ARE TO BE CONSTRUCTED WITH ADEQUATE SUBSOIL DRAINAGE.

#### CONCRETE PAVEMENT

- C1. SUBGRADE SHALL BE PREPARED AS OUTLINED IN EARTHWORKS.
- C2. PROVIDE JOINTING AT MINIMUM 6000 MAX. INTERVALS OR AS OTHERWISE SPECIFIED IN THE DRAWINGS.
- C3. CONCRETE SHALL COMPRISE A MIN. COMPRESSIVE STRENGTH OF 32MPa AT 28 DAYS IN ACCORDANCE WITH THE RELEVANT LOCAL AUTHORITY SPECIFICATION. UNO.
- C4. ANY SUB-BASE MATERIAL SHALL BE COMPACTED AS OUTLINED IN EARTHWORKS.
- C5. CONCRETE KERB AND GUTTER SHALL COMPRISE A MINIMUM COMPRESSIVE STRENGTH OF 25MPa. UNO.
- C6. CONCRETE WORKS ARE TO BE CURED BY ONE OF THE FOLLOWING MEANS:
  - i) WETTING TWICE DAILY FOR THE FIRST THREE DAYS.
  - ii) USING AN APPROVED CURING COMPOUNDED FOR A MINIMUM OF 7 DAYS COMMENCING IMMEDIATELY AFTER POURING.

#### FLEXIBLE PAVEMENT NOTES

- F1. SUBGRADE SHALL BE PREPARED AS OUTLINED IN EARTHWORKS.
- F2. PAVEMENT MATERIAL SHALL CONSIST OF APPROVED OR RIPPED SANDSTONE, NATURAL GRAVEL OR FINE CRUSH ROCK AS PER THE RELEVANT COUNCIL AUTHORITY SPECIFICATION.
- F3. PAVEMENT MATERIALS SHALL BE SPREAD IN LAYERS NOT EXCEEDING 150 AND NOT LESS 75 COMPACTED THICKNESS.
- F4. PAVEMENT MATERIALS SHALL BE SIZED AND OF A STANDARD OUTLINED IN AS1141.
- F5. CRUSHED OR RIPPED SANDSTONE SHALL BE MINUS 75 NOMINAL SIZE DERIVED FROM SOUND, CLEAN SANDSTONE FREE FROM OVERBURDEN, CLAY SEAMS, SHALE AND OTHER DELETERIOUS MATERIAL.
- F6. PAVEMENT MATERIALS SHALL BE COMPACTED BY SUITABLE MEANS TO SATISFY THE FOLLOWING MINIMUM SPECIFICATIONS (AS PER AS1289.2)
 

DESCRIPTION	MEDIUM DENSITY RATIO
SUB-BASE	98% MOD
BASE COURSE	98% MOD
ASPHALTIC CONCRETE	97% MOD

 AND SUBJECT TO THE RELEVANT LOCAL AUTHORITY CONSTRUCTION SPECIFICATION.
- F7. TESTING FOR EACH LAYER SHALL BE UNDERTAKEN BY A N.A.T.A. REGISTERED LABORATORY IN ACCORDANCE WITH AS1289, AT NOT MORE THAN 50m INTERVALS AND A MINIMUM OF TWO PER LAYER. FURTHER FREQUENCY OF TESTING SHALL BE NO LESS THAN THAT REQUIRED BY AS3978.

#### PAVED AREAS NOTES

- A1. SUBGRADE SHALL BE PREPARED AS OUTLINED IN EARTHWORKS.
- A2. ALL PAVERS ARE TO BE PLACED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATION.
- A3. TRAFFICABLE AREAS:
  - SUB-BASE TO BE 150 COMPACTED THICKNESS DGS75
  - SUB-BASE TO BE SUITABLY COMPACTED TO MEDIUM DENSITY 98% MOD.
  - SUB-BASE TO EXTEND AT LEAST 200 BEYOND PAVED SURFACE.
  - PAVERS TO BE 80 THICK INTERLOCKING PAVERS ON 50 SAND BEDDING.
- A4. NON TRAFFICABLE AREAS:
  - SUB BASE AS PER TRAFFICABLE AREAS
  - PAVERS TO BE 60 INTERLOCKING PAVERS ON 50 SAND BEDDING (UNO).

#### EROSION AND SEDIMENT NOTES

- B1. THIS PLAN TO BE READ IN CONJUNCTION WITH EROSION AND SEDIMENT CONTROL DETAILS AS ATTACHED.
- B2. THE CONTRACTOR SHALL IMPLEMENT ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES AS NECESSARY AND TO THE SATISFACTION OF THE RELEVANT LOCAL AUTHORITY PRIOR TO THE COMMENCEMENT OF AND DURING CONSTRUCTION. NO DISTURBANCE TO THE SITE SHALL BE PERMITTED OTHER THAN IN THE IMMEDIATE AREA OF THE WORKS AND NO MATERIAL SHALL BE REMOVED FROM THE SITE WITHOUT THE RELEVANT LOCAL AUTHORITY APPROVAL. ALL EROSION AND SEDIMENT CONTROL DEVICES TO BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH STANDARDS OUTLINED IN NSW DEPARTMENT OF HOUSING'S "MANAGING URBAN STORMWATER - SOILS AND CONSTRUCTIONS".
- B3. TOPSOIL SHALL BE STRIPPED AND STOCKPILED OUTSIDE HAZARD AREAS SUCH AS DRAINAGE LINES. THIS TOPSOIL SHALL BE RESPREAD LATER ON AREAS TO BE REVEGETATED AND STABILISED ONLY. (I.E. ALL FOOTPATHS, BATTERS, SITE REGARDING AREAS, BASINS AND CATCHDRAINS). TOPSOIL SHALL NOT BE RESPREAD ON ANY OTHER AREAS UNLESS SPECIFICALLY INSTRUCTED BY THE SUPERINTENDENT. IF THEY ARE TO REMAIN FOR LONGER THAN ONE MONTH STOCKPILES SHALL BE PROTECTED FROM EROSION BY COVERING THEM WITH A MULCH AND HYDROSEEDING AND, IF NECESSARY, BY LOCATING BANKS OR DRAINS DOWNSTREAM OF A STOCKPILE TO RETARD SILT LADEN RUNOFF.
- B4. THE CONTRACTOR SHALL REGULARLY MAINTAIN ALL EROSION AND SEDIMENT CONTROL DEVICES AND REMOVE ACCUMULATED SILT FROM SUCH DEVICES. SUCH THAT MORE THAN 60% OF THEIR CAPACITY IS LOST. ALL THE SILT IS TO BE PLACED OUTSIDE THE LIMIT OF WORKS. THE PERIOD FOR MAINTAINING THESE DEVICES SHALL BE AT LEAST UNTIL ALL DISTURBED AREAS ARE REVEGETATED AND FURTHER AS MAY BE DIRECTED BY THE SUPERINTENDENT OR COUNCIL.
- B5. LAY TURF STRIP (MIN 300 WIDE) ON 100 TOPSOIL BEHIND ALL KERB WITH 1000 LONG RETURNS EVERY 6000 AND AROUND STRUCTURES IMMEDIATELY AFTER BACKFILLING AS PER THE RELEVANT LOCAL AUTHORITY SPECIFICATION.
- B6. THE CONTRACTOR SHALL GRASS SEED ALL DISTURBED AREAS WITH AN APPROVED MIX AS SOON AS PRACTICABLE AFTER COMPLETION OF EARTHWORKS AND REGRADING.
- B7. VEHICULAR TRAFFIC SHALL BE CONTROLLED DURING CONSTRUCTION CONFINING ACCESS WHERE POSSIBLE TO NOMINATED STABILISED ACCESS POINTS.
- B8. WHEN ANY DEVICES ARE TO BE HANDED OVER TO COUNCIL, THEY SHALL BE IN CLEAN AND STABLE CONDITION.
- B9. THE CONTRACTOR SHALL IMPLEMENT DUST CONTROL BY REGULAR WETTING DOWN (BUT NOT SATURATING) DISTURBED AREA.
- B10. PROVIDE AND MAINTAIN SILT TRAPS AROUND ALL SURFACE INLET PITS UNTIL CATCHMENT IS REVEGETATED OR PAVED.
- B11. REVEGETATE ALL TRENCHES IMMEDIATELY UPON COMPLETION OF BACKFILLING.
- B12. ALL DRAINAGE PIPE INLETS TO BE CAPPED UNTIL:
  - DOWNPIPES CONNECTED
  - PITS CONSTRUCTED AND PROTECTED WITH SILT BARRIER

#### CONCRETE STRUCTURES NOTES

- S1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3600 CURRENT EDITION WITH AMENDMENTS, EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- S2. CONCRETE COMPONENTS AND QUALITY SHALL BE AS FOLLOWS. UNO:

ELEMENT	SLUMP mm	MAX. SIZE AGG. mm	CEMENT TYPE	f <sub>c</sub> AT 28 DAYS - MPa	ADMIXTURE
FOOTINGS	80	20	A	25	-
PIERS & CAPS	80	20	A	25	-
SLABS ON GROUND	80	20	A	32	-
SUSPENDED SLABS	80	20	A	32	-
PITS	80	20	A	25	-

- S3. MINIMUM CLEAR CONCRETE COVER TO REINFORCEMENT INCLUDING TIES AND STIRRUPS SHALL BE AS FOLLOWS UNO.

EXPOSURE CLASSIFICATION	MINIMUM COVER (mm)				
	CONCRETE STRENGTH (f <sub>c</sub> )				
	20 MPa	25 MPa	32 MPa	40 MPa	>50 MPa
A1	20	20	20	20	20
A2	(50)	30	25	20	20
B1	-	(60)	40	30	25
B2	-	-	(65)	45	35
C	-	-	-	(70)	50

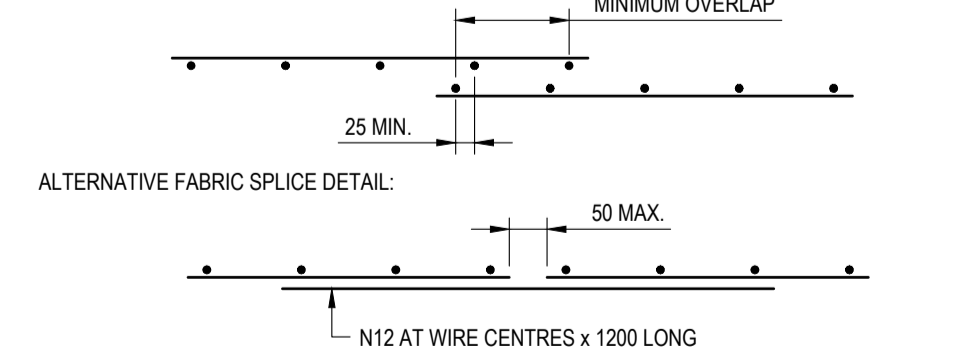
FOR BRACKETED FIGURES REFER TO AS 3600 CURRENT EDITION TABLE 4.10.3.2

- S4. MINIMUM COVER FOR FIRE RESISTANCE LEVEL (FRL) SHALL BE AS FOLLOWS:

FRL	MINIMUM ELEMENT WIDTH OR THICKNESS / MIN COVER (mm)			
	BEAM	SLAB	COLUMN	WALL
60	125 / 30	80 / 20	200 / 20	80 / 20
90	150 / 45	100 / 25	250 / 35	100 / 35
120	200 / 55	120 / 30	300 / 45	120 / 40
180	240 / 70	150 / 45	400 / 60	150 / 45
240	270 / 80	170 / 55	450 / 70	170 / 50

NOTE : 1. REFER TO AS 3600 CURRENT EDITION FOR REDUCED COVERS IF GREATER ELEMENT THICKNESSES ARE ADOPTED FOR BEAMS & COLUMNS.  
2. COVER IS MEASURED TO THE MAIN REINFORCEMENT

- S5. COVER TO REINFORCEMENT SHALL BE OBTAINED BY THE USE OF APPROVED BAR CHAIRS. ALL CHAIRS SHALL BE SPACED AT 1000 CTS MAXIMUM.
- S6. ALL CONCRETE SHALL BE MECHANICALLY VIBRATED. VIBRATORS SHALL NOT BE USED TO SPREAD CONCRETE.
- S7. SIZES OF CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES.
- S8. NO HOLES OR CHASES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT THE PRIOR APPROVAL OF THE ENGINEER.
- S9. CONSTRUCTION JOINTS WHERE NOT SHOWN SHALL BE LOCATED TO APPROVAL OF THE ENGINEER. ALL CONSTRUCTION JOINTS SHALL BE SCABBLED OVER THE WHOLE FACE AND ANY UNSOUND MATERIAL REMOVED.
- S10. REINFORCEMENT IS REPRESENTED DIAGMATICALLY; IT IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.
- S11. SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN OR AS APPROVED BY THE ENGINEER. WHERE THE LAP LENGTH IS NOT SHOWN IT SHALL BE SUFFICIENT TO DEVELOP THE FULL STRENGTH OF THE REINFORCEMENT AS SPECIFIED IN AS3600. COGS AND HOOKS SHALL BE STANDARD UNLESS SHOWN OTHERWISE.
- S12. WELDING OF REINFORCEMENT WILL NOT BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE ENGINEER.
- S13. PIPES OR CONDUITS SHALL NOT BE PLACED WITHIN THE CONCRETE COVER TO REINFORCEMENT WITHOUT THE APPROVAL OF THE ENGINEER.
- S14. REINFORCEMENT SYMBOLS:
  - N - DENOTES DEFORMED GRADE 500 NORMAL DUCTILITY REINFORCING BARS TO AS/NZS 4671.
  - R - DENOTES PLAIN ROUND GRADE 250 NORMAL DUCTILITY REINFORCING BARS TO AS/NZS 4671.
  - SL - DENOTES DEFORMED GRADE 500 LOW DUCTILITY REINFORCING MESH TO AS/NZS 4671.
  - RL - DENOTES DEFORMED GRADE 500 LOW DUCTILITY REINFORCING MESH TO AS/NZS 4671.
  - L-TM - DENOTES DEFORMED GRADE 500 LOW DUCTILITY TRENCH MESH TO AS/NZS 4671.
- S15. ALL REINFORCING FABRIC SHALL COMPLY WITH AS1303 AND AS1304 AND SHALL BE SUPPLIED IN FLAT SHEETS.
- S16. SPLICES IN FABRIC: THE OUTERMOST TRANSVERSE WIRES SHALL BE OVERLAPPED BY AT LEAST THE SPACING OF THESE TRANSVERSE WIRES PLUS 25 mm.



- S17. EXPOSED CORNERS SHALL BE 20 mm CHAMFERED UNO.
- S18. ALL REINFORCEMENT SHALL BE INSPECTED BY THE SUPERINTENDENT OR ENGINEER PRIOR TO PLACING CONCRETE.
- S19. ALL SLAB CONCRETE TO BE CURED IN AN APPROVED MANNER FOR A MINIMUM OF 7 DAYS.
- S20. ALL FORMWORK AND PROPS FOR SLABS AND BEAMS SHALL BE REMOVED BEFORE CONSTRUCTION OF ANY MASONRY WALLS OR PARTITIONS ON THE FLOOR.
- S21. ALL ABBREVIATIONS ARE IN ACCORDANCE WITH AS1100.
- S22. FORMWORK SHALL NOT BE STRIPPED UNTIL CONCRETE HAS ACHIEVED A MINIMUM STRENGTH OF 20 MPa. THE CONCRETE SLAB AND BEAMS SHALL BE TEMPORARILY BACK PROPPED UNTIL THE CONCRETE HAS ACHIEVED 28 DAY STRENGTH AND ANY PROPPING TO HIGHER LEVEL FORMS HAVE BEEN REMOVED.
- S23. WHERE A SUSPENDED SLAB IS TO BE SUPPORTED OFF A SUSPENDED SLAB BELOW, WRITTEN APPROVAL SHALL BE OBTAINED FROM THE ENGINEER PRIOR TO ANY SITE WORKS.

#### MASONRY

- M1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3700.
- M2. THE DESIGN STRENGTH OF MASONRY SHALL BE AS FOLLOWS U.N.O. :

EXPOSURE CLASSIFICATION TO AS 3600	MASONRY COMPRESSIVE STRENGTH MPa (f <sub>m</sub> )	MASONRY SALT RESISTANCE GRADE	DURABILITY CLASSIFICATION OF BUILT IN COMPONENTS	MORTAR MIX GP PORTLAND CEMENT : LIME : SAND	f <sub>c</sub> MPa
A1 / A2	> 6.3	General Purpose	R3 (Galvanised)	1.0 : 1.0 : 6.0	2.8
B1	> 6.3	General Purpose	R3 (Galvanised)	1.0 : 1.0 : 6.0	2.8
B2	> 6.7	Exposure	R4 (Stainless)	1.0 : 0.5 : 4.5	2.8

- M3. ALL MASONRY WALLS SUPPORTING SLABS AND BEAMS SHALL HAVE A PRE-GREASED TWO LAYER GALVANISED STEEL SLIP JOINT BETWEEN CONCRETE AND MASONRY.

- M4. ALL MASONRY WALLS SUPPORTING OR SUPPORTED BY CONCRETE FLOORS SHALL BE PROVIDED WITH VERTICAL JOINTS TO MATCH ANY CONTROL JOINTS IN THE CONCRETE.

- M5. NON LOAD BEARING WALLS SHALL BE SEPARATED FROM CONCRETE ABOVE BY 20 mm THICK CLOSED CELL POLYETHYLENE STRIP.

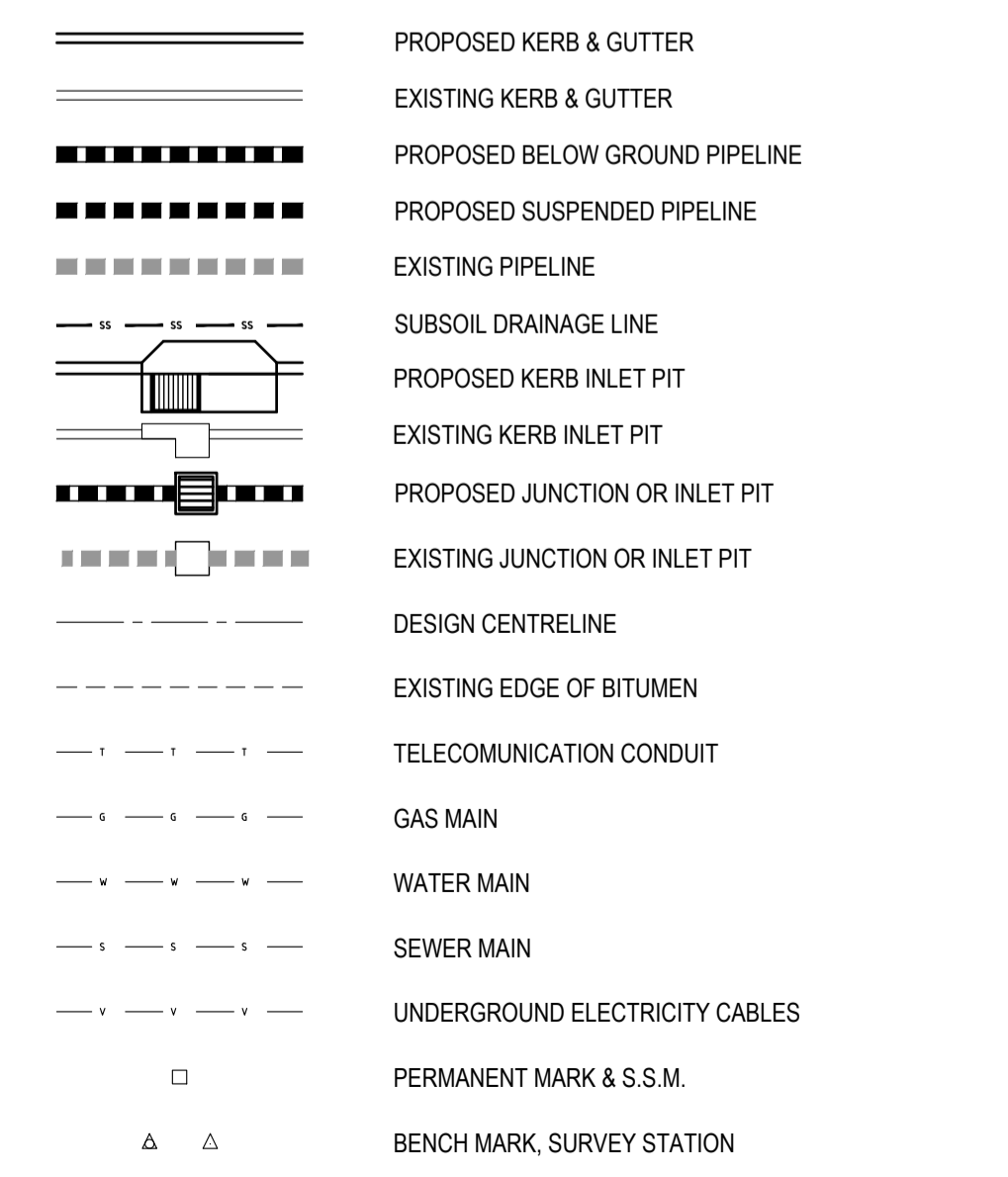
- M6. MASONRY SHALL BE ARTICULATED IN ACCORDANCE WITH TECHNICAL NOTE 61 FROM THE CEMENT AND CONCRETE ASSOCIATION OF AUSTRALIA. VERTICAL CONTROL JOINTS SHALL NOT EXCEED 5 METRES MAXIMUM CENTRES, AND 4 METRES MAXIMUM FROM CORNERS IN MASONRY WALLS, AND BETWEEN NEW & EXISTING BRICKWORK.

- M7. MASONRY RETAINING WALLS ARE TO BE BACKFILLED WITH EITHER OF THE FOLLOWING MATERIAL:
  - COARSE GRAINED SOIL WITH LOW SILT CONTENT
  - RESIDUAL SOIL CONTAINING STONES
  - FINE SILTY SAND
  - GRANULAR MATERIALS WITH LOW CLAY CONTENT

#### BLOCKWORK

- B1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3700.
- B2. REINFORCED CONCRETE BLOCKWORK SHALL COMPLY WITH THE FOLLOWING. UNO:
  - BLOCKS : GRADE 15 CONFORMING TO AS1500.
  - MORTAR : 1 CEMENT / 0.25 LIME / 3 SAND.
  - PROVIDE CLEANOUT HOLES AT BASE OF WALL & ROD CORE HOLES TO REMOVE PROTRUDING MORTAR FINIS.
  - CORE FILLING : f<sub>c</sub> = 20 MPa, 10 AGG, 230 SLUMP +/- 30 mm.
  - COVER : 55 mm MIN. FROM OUTSIDE OF BLOCKWORK.
- B3. BACKFILL TO RETAINING WALLS TO BE FREE DRAINING GRANULAR MATERIAL, UNO. PROVIDE SUBSOIL DRAIN BEHIND WALL AND AT WEEP HOLES.
- B4. VERTICAL CONTROL JOINTS SHALL BE PROVIDED AT 10 m MAX. CENTRES.
- B5. NO ADMIXTURES SHALL BE USED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

#### STANDARD LINE TYPES AND SYMBOLS:



#### SCHEDULE OF DRAWINGS

SHEET No	DESCRIPTION
C01	GENERAL NOTES
C02	SEDIMENT & EROSION CONTROL PLAN
C03	STORMWATER CATCHMENT AREA PLAN
C04	STORMWATER DRAINAGE PLAN
C05	EXTERNAL PAVEMENT PLAN AND DETAILS
C06	STORMWATER DETAILS SHEET 1 OF 2
C07	STORMWATER DETAILS SHEET 2 OF 2
C08	BULK EARTHWORKS CUT AND FILL PLAN
C09	SITE CROSS SECTIONS SHEET 1 OF 2
C10	SITE CROSS SECTIONS SHEET 2 OF 2

**FOR APPROVAL**  
NOT TO BE USED FOR CONSTRUCTION PURPOSES

REVISION	DATE	AMENDMENT DESCRIPTION
B	10.02.21	ISSUED FOR APPROVAL
A	05.02.21	ISSUED FOR APPROVAL

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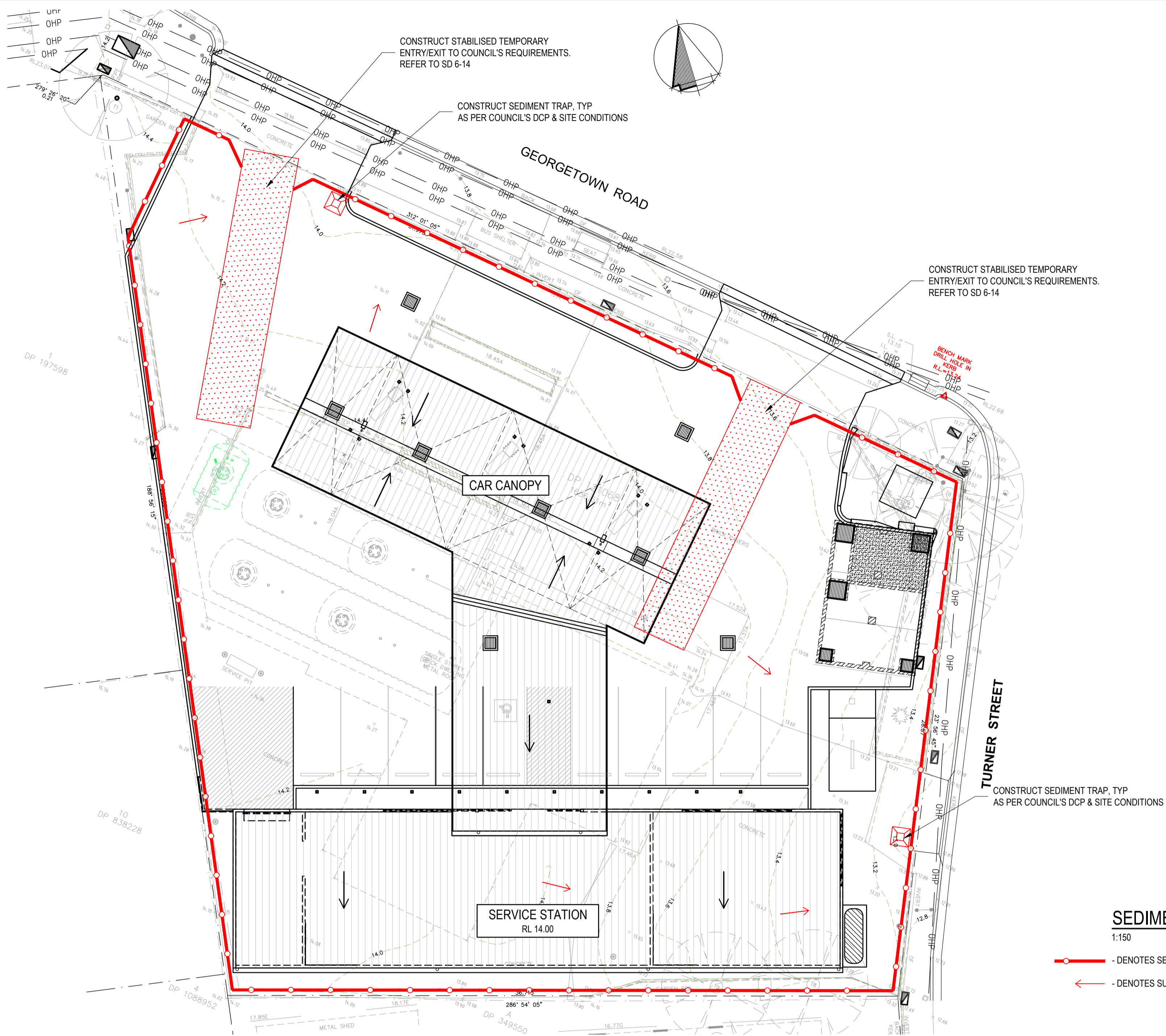
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**PROPOSED DEVELOPMENT**  
42 Georgetown Road, Georgetown  
For GWH Build Pty Ltd

GENERAL NOTES

DESIGN	DRAWN	DATE	PROJECT No
SWH	RCL	JAN 2021	10190
CHECKED	APPROVED	SCALE	DRG No.
			C01 - B

AT ORIGINAL SIZE



**SEDIMENT & EROSION CONTROL PLAN**

1:150

- - DENOTES SEDIMENT FENCE
- - DENOTES SURFACE WATER FLOW

**SEDIMENT AND EROSION CONTROL NOTES**

SEDIMENT AND EROSION CONTROL SHALL BE EFFECTIVELY MAINTAINED AT ALL TIMES DURING THE COURSE OF CONSTRUCTION AND SHALL NOT BE REMOVED UNTIL THE SITE HAS BEEN STABILISED OR LANDSCAPED TO THE SUPERINTENDENT'S SATISFACTION.

A SINGLE ALL WEATHER ACCESS WAY WILL BE PROVIDED AT THE FRONT OF THE PROPERTY CONSISTING OF 50-75 AGGREGATE OR SIMILAR MATERIAL AT A MINIMUM THICKNESS OF 150 LAID OVER NEEDLE-PUNCHED GEOTEXTILE FABRIC AND CONSTRUCTED PRIOR TO COMMENCEMENT OF WORKS.

THE CONTRACTOR SHALL ENSURE THAT NO SPOIL OR FILL ENCROACHES UPON ADJACENT AREAS FOR THE DURATION OF WORKS.

THE CONTRACTOR SHALL ENSURE THAT KERB INLETS AND DRAINS RECEIVING STORMWATER SHALL BE PROTECTED AT ALL TIMES DURING DEVELOPMENT. KERB INLET SEDIMENT TRAPS SHALL BE INSTALLED ALONG THE IMMEDIATE VICINITY ALONG THE STREET FRONTAGE.

ALL TOPSOIL STRIPPED FROM THE SITE AND STOCKPILED DOES NOT INTERFERE WITH DRAINAGE LINES AND STORMWATER INLETS AND WILL BE SUITABLY COVERED WITH AN IMPERVIOUS MEMBRANE MATERIAL AND SCREENED BY SEDIMENT FENCING.

**SOIL CONSERVATION NOTE:**

PRIOR TO COMMENCEMENT OF CONSTRUCTION PROVIDE 'SEDIMENT FENCE', 'SEDIMENT TRAP' AND WASHOUT AREA TO ENSURE THE CAPTURE OF WATER BORNE MATERIAL GENERATED FROM THE SITE.

MAINTAIN THE ABOVE DURING THE COURSE OF CONSTRUCTION, AND CLEAR THE 'SEDIMENT TRAP' AFTER EACH STORM.

**SEDIMENT TRAP**

1000 x 1000 WIDE 500 DEEP PIT, LOCATED AT THE LOWEST POINT TO THE TRAP SEDIMENT AND IN ACCORDANCE WITH LOCAL COUNCIL'S DCP AND SITE CONDITIONS.

**SEDIMENT FENCE**

PROVIDE 'SEDIMENT FENCE' ON DOWN SLOPE BOUNDARY AS SHOWN ON PLAN. FABRIC TO BE BURIED BELOW GROUND AT LOWER EDGE. REFER TO SD 6-8

**BUILDING MATERIAL STOCKPILES**

ALL STOCKPILES OF BUILDING MATERIAL SUCH AS SAND AND SOIL MUST BE PROTECTED TO PREVENT SCOUR AND EROSION.

THEY SHOULD NEVER BE PLACED IN THE STREET GUTTER WHERE THEY WILL WASH AWAY WITH THE FIRST RAINSTORM. REFER TO SD 4-1

**GENERAL NOTES**

THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH OTHER CONSULTANTS' DRAWINGS AND SPECIFICATIONS AND WITH OTHER SUCH WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCY SHALL BE REFERRED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

ALL DIMENSIONS ARE IN MILLIMETRES & ALL LEVELS ARE IN METRES, UNO (UNLESS NOTED OTHERWISE).

NO DIMENSION SHALL BE OBTAINED BY SCALING THE DRAWINGS.

ALL LEVELS AND SETTING OUT DIMENSIONS SHOWN ON THE DRAWINGS SHALL BE CHECKED ON SITE PRIOR TO THE COMMENCEMENT OF THE WORK.

DURING EXCAVATION WORK THE STRUCTURE SHALL BE MAINTAINED IN A STABLE AND NO PART SHALL BE OVERSTRESSED.

ALL WORK IS TO BE UNDERTAKEN IN ACCORDANCE WITH THE DETAILS SHOWN ON THE DRAWINGS & THE SPECIFICATION.

EXISTING SERVICES WHERE SHOWN HAVE BEEN PLOTTED FROM SUPPLIED DATA AND SUCH THEIR ACCURACY CAN NOT BE GUARANTEED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF WORK.

ALL SERVICE TRENCHES UNDER VEHICULAR PAVEMENTS SHALL BE BACK FILLED IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL COUNCIL.

ALL TRENCH BACK FILL MATERIAL SHALL BE COMPACTED TO THE SAME DENSITY AS THE ADJACENT MATERIAL.

ON COMPLETION OF STORMWATER INSTALLATION, ALL DISTURBED AREAS MUST BE RESTORED TO ORIGINAL CONDITION, INCLUDING KERBS, FOOTPATHS, CONCRETE AREAS, GRAVEL AND GRASSED AREAS AND ROAD PAVEMENTS, UNLESS DIRECTED OTHERWISE.

CONTRACTOR TO OBTAIN ALL AUTHORITY APPROVALS UNLESS DIRECTED OTHERWISE.

**STORMWATER DRAINAGE**

THE STORMWATER DRAINAGE DESIGN HAS BEEN CARRIED OUT IN ACCORDANCE WITH AS/NZS 3500.3 - 1990 'STORMWATER DRAINAGE' & AS/NZS 3500.3.2-1998 'STORMWATER DRAINAGE - ACCEPTABLE SOLUTIONS'.

ANY VARIATIONS TO THE NOMINATED LEVELS SHALL BE REFERRED TO ENGINEER IMMEDIATELY.

ANY VARIATIONS TO SPECIFIED PRODUCTS OR DETAILS SHALL BE REFERRED TO THE ENGINEER FOR APPROVAL.

DOWN PIPES SHALL BE A MINIMUM OF DN100 SW GRADE UPVC OR 100X100 COLORBOND/ZINCALUME STEEL UNO.

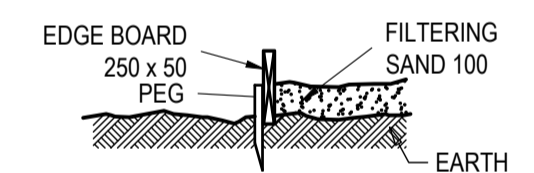
BOX COLORBOND OR ZINCALUME STEEL. GUTTERS SHALL BE A MINIMUM OF 450 WIDE X 150 DEEP.

EAVES GUTTERS SHALL BE A MINIMUM OF 125 WIDE X 100 DEEP (OR OF EQUIVALENT AREA) COLORBOND OR ZINCALUME STEEL.

SUBSOIL DRAINAGE SHALL BE PROVIDED TO ALL RETAINING WALLS & EMBANKMENTS, WITH THE LINES FEEDING INTO THE STORMWATER DRAINAGE SYSTEM.

**WASHOUT AREA**

TO BE 1800 x 1800 ALLOCATED FOR THE WASHING OF TOOL & EQUIPMENT.



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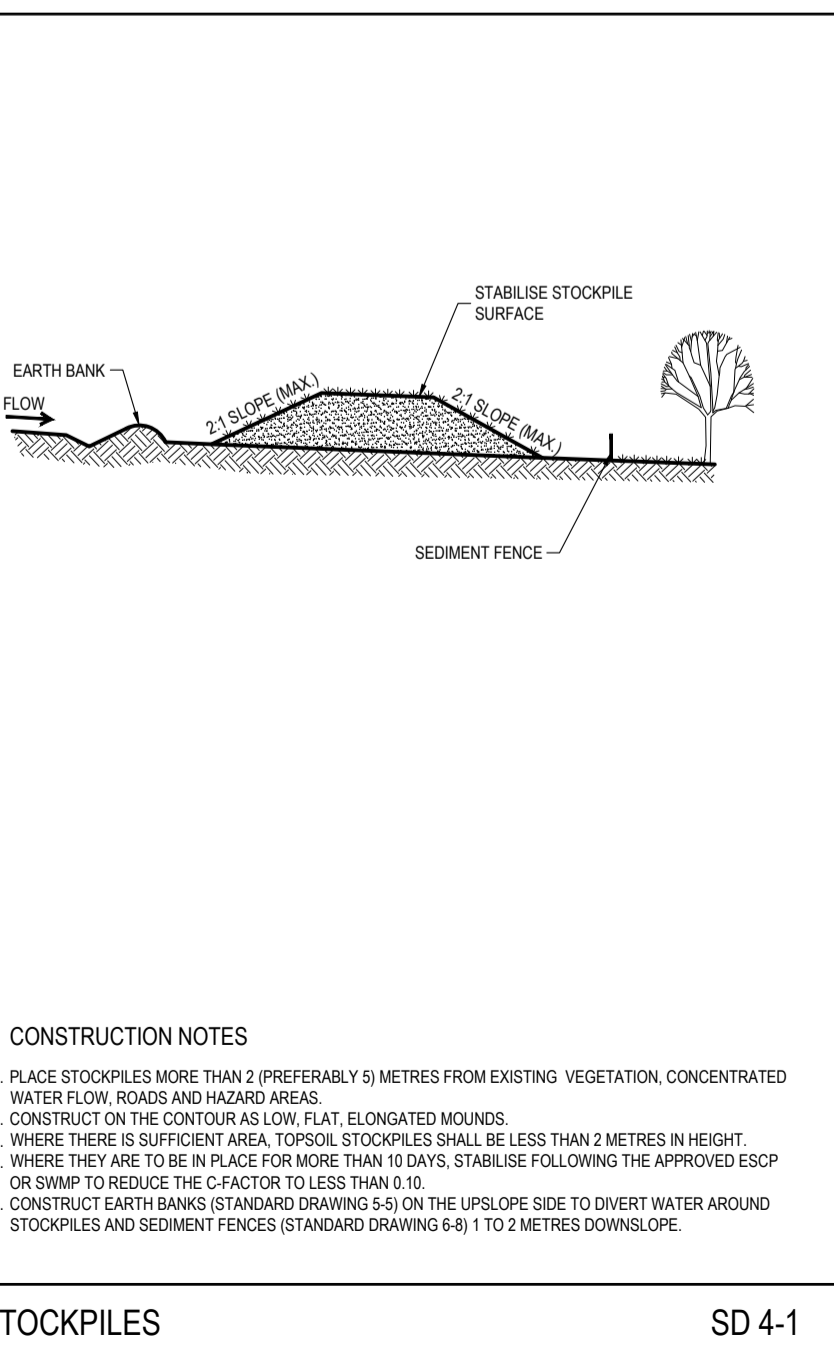
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 For GWH Build Pty Ltd

**SEDIMENT & EROSION CONTROL PLAN**

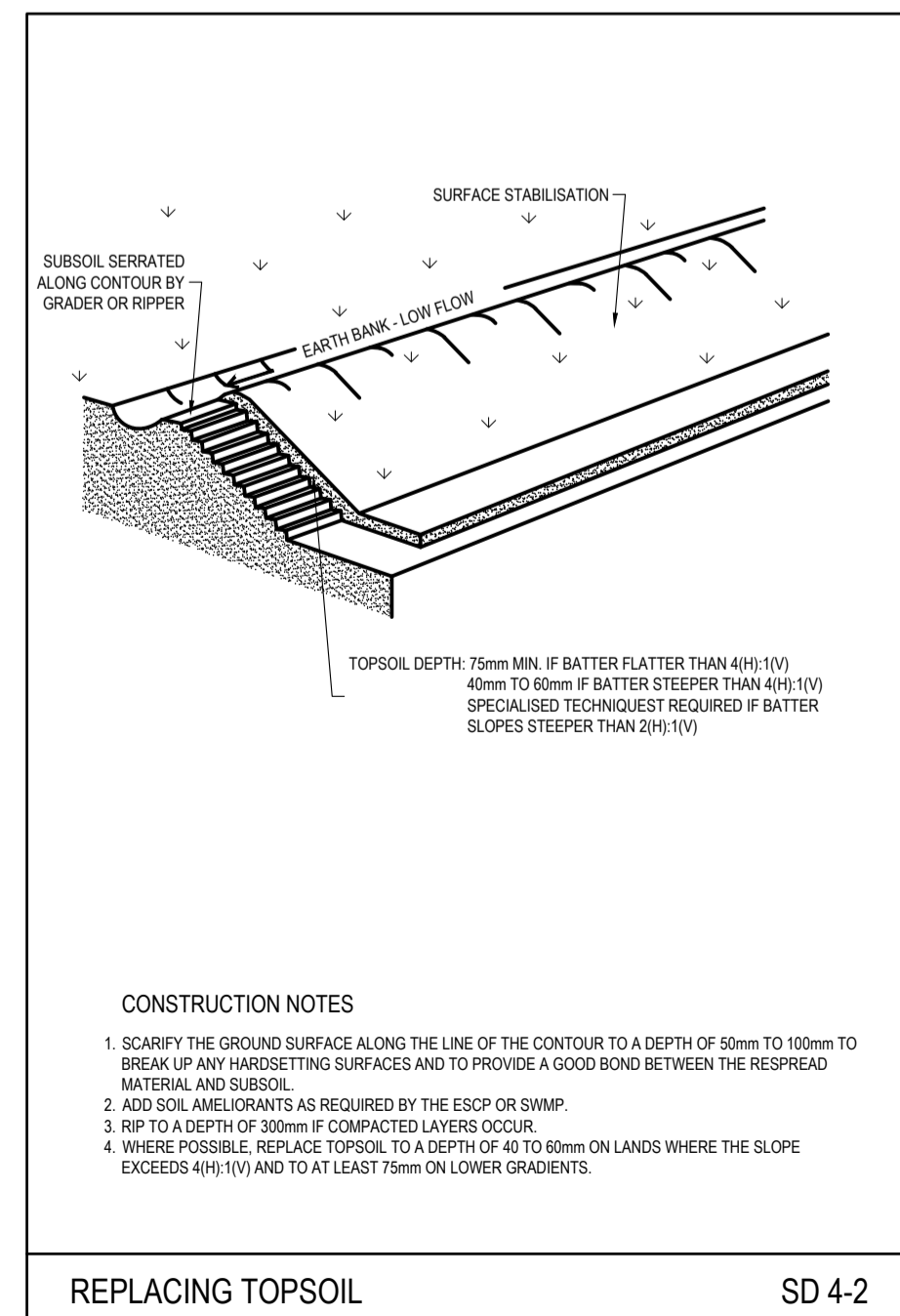
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		1:150	C02 - B



**CONSTRUCTION NOTES**

- PLACE STOCKPILES MORE THAN 2 (PREFERABLY 6) METRES FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW, ROADS AND HAZARDOUS AREAS.
- CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.
- WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2 METRES IN HEIGHT.
- WHERE THEY ARE TO BE IN PLACE FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE APPROVED ESCP OR SWMP TO REDUCE THE C-FACTOR TO LESS THAN 0.10.
- CONSTRUCT EARTH BANKS (STANDARD DRAWING S-5) ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDIMENT FENCES (STANDARD DRAWING S-6) TO 2 METRES DOWN SLOPE.

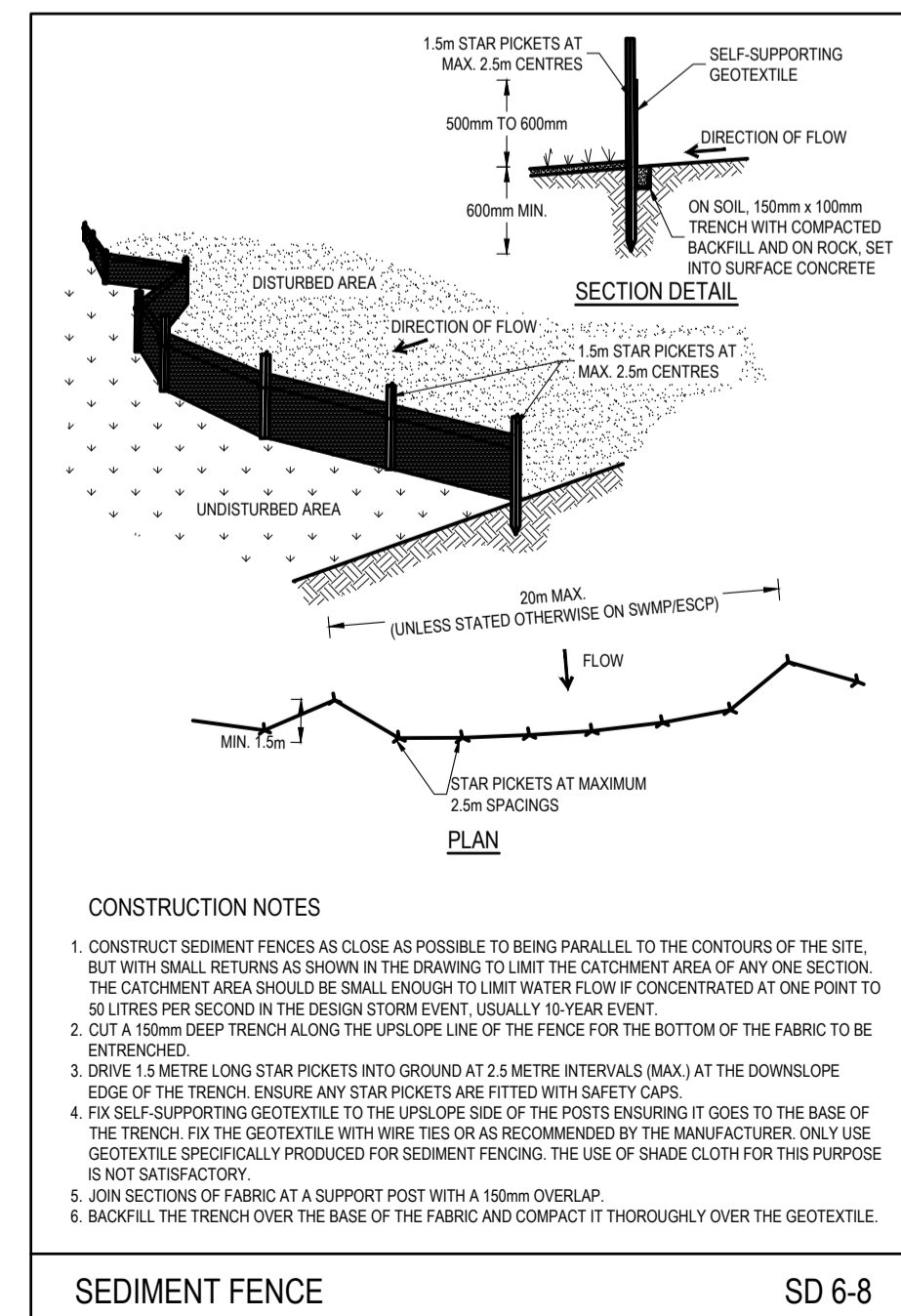
STOCKPILES SD 4-1



**CONSTRUCTION NOTES**

- SCARIFY THE GROUND SURFACE ALONG THE LINE OF THE CONTOUR TO A DEPTH OF 50mm TO 100mm TO BREAK UP ANY HARDETTING SURFACES AND TO PROVIDE A GOOD BOND BETWEEN THE RESURFACED MATERIAL AND SUBSOIL.
- ADD SOIL AMELIORANTS AS REQUIRED BY THE ESCP OR SWMP.
- RIP TO A DEPTH OF 300mm IF COMPACTED LAYERS OCCUR.
- WHERE POSSIBLE, REPLACE TOPSOIL TO A DEPTH OF 40 TO 60mm ON LANDS WHERE THE SLOPE EXCEEDS 40% (1:1) AND TO AT LEAST 75mm ON LOWER GRADIENTS.

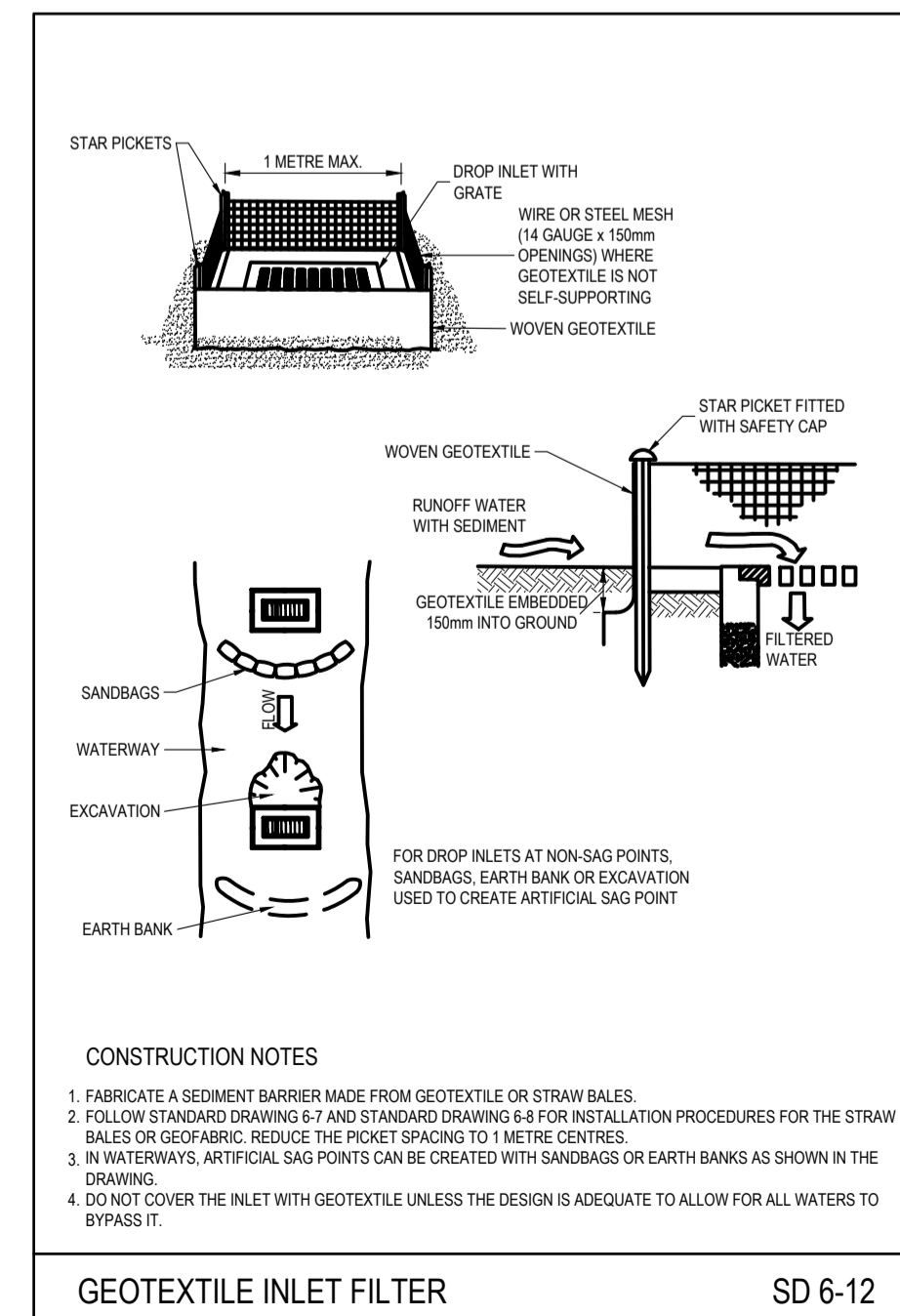
REPLACING TOPSOIL SD 4-2



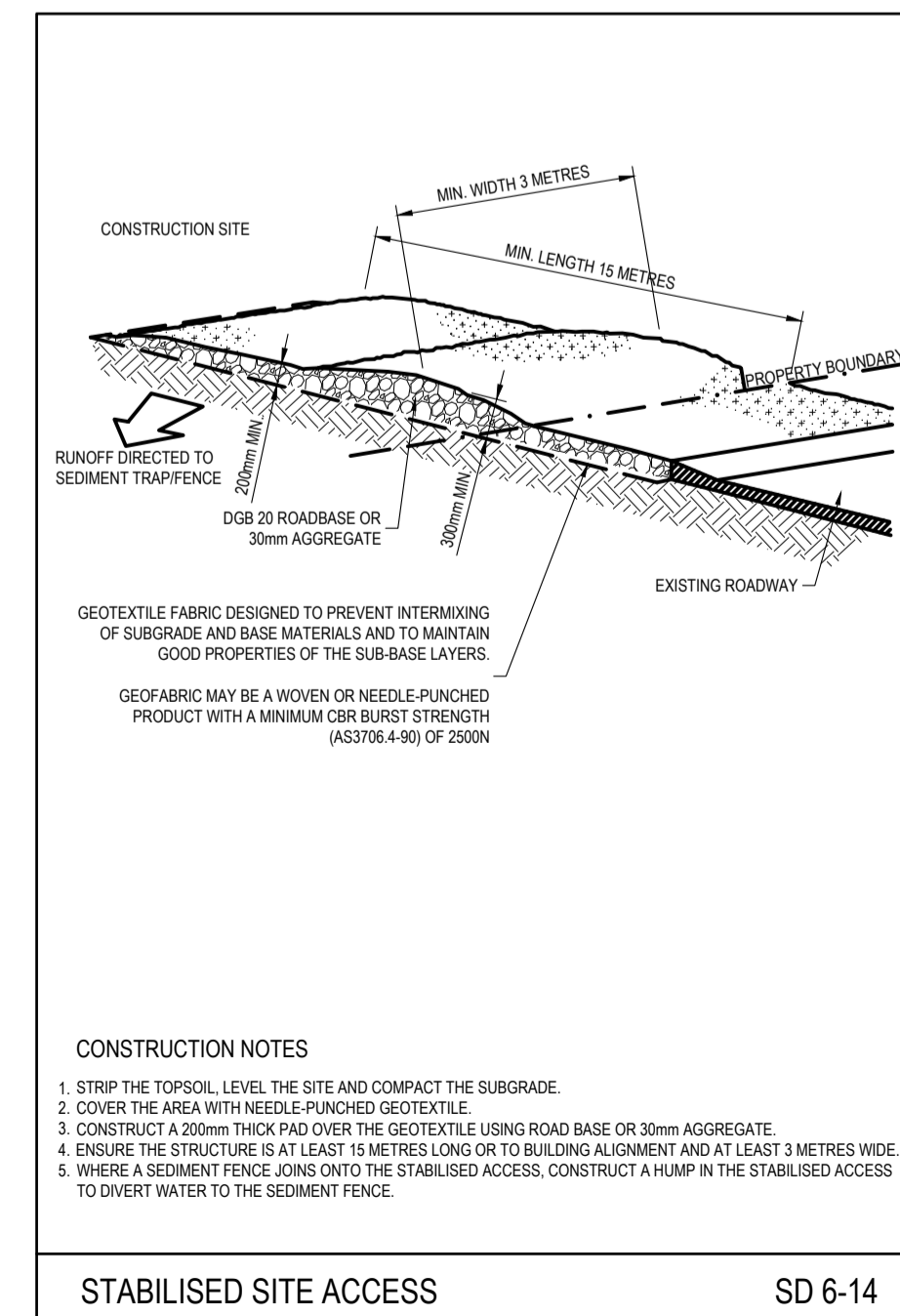
**CONSTRUCTION NOTES**

- CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER SECOND IN THE DESIGN STORM EVENT, USUALLY 10-YEAR EVENT.
- CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED.
- DRIVE 1.5 METRE LONG STAR PICKETS INTO GROUND AT 2.5 METRE INTERVALS (MAX.) AT THE DOWNSLOPE EDGE OF THE TRENCH ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
- FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
- BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

SEDIMENT FENCE SD 6-8



GEOTEXTILE INLET FILTER SD 6-12

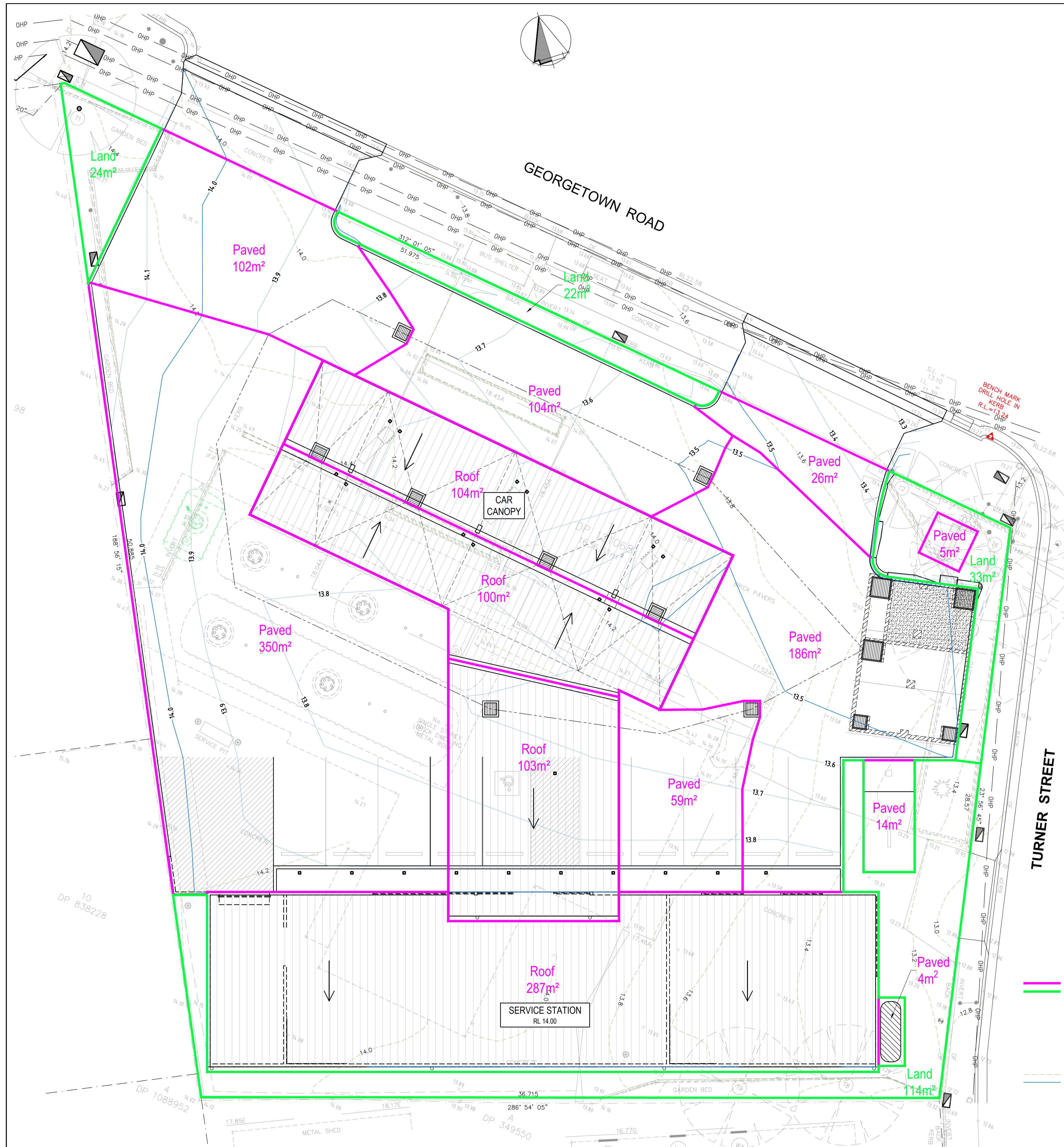


**CONSTRUCTION NOTES**

- STRIP THE TOPSOIL, LEVEL THE SITE AND COMPACT THE SUBGRADE.
- COVER THE AREA WITH NEEDLE-PUNCHED GEOTEXTILE.
- CONSTRUCT A 200mm THICK PAD OVER THE GEOTEXTILE USING ROAD BASE OR 30mm AGGREGATE.
- ENSURE THE STRUCTURE IS AT LEAST 15 METRES LONG OR TO BUILDING ALIGNMENT AND AT LEAST 3 METRES WIDE.
- WHERE A SEDIMENT FENCE JOINS ONTO THE STABILISED ACCESS, CONSTRUCT A HUMP IN THE STABILISED ACCESS TO DIVERT WATER TO THE SEDIMENT FENCE.

STABILISED SITE ACCESS SD 6-14

AT ORIGINAL SIZE



**STORMWATER DRAINAGE STRATEGY**

- ALL GUTTERS & DOWNPIPES ARE DESIGNED TO ACCEPT A 1:20 YEAR ARI STORM EVENT.
- BOX GUTTERS & DOWNPIPES ARE DESIGNED TO ACCEPT A 1:100 YEAR ARI STORM EVENT.
- ALL PITS & PIPES ARE DESIGNED TO ACCEPT A 1:20 YEAR ARI STORM EVENT.
- DESIGN RAINFALL INTENSITIES:  
1:20 YEAR, 5 MIN = 221 mm/hr  
1:100 YEAR, 5 MIN = 308 mm/hr
- ALL PIPES MUST HAVE A MIN. 1.0% FALL UNO.
- THE FOLLOWING SOILS HAVE BEEN DESIGNED FOR THIS DEVELOPMENT
  - RAINWATER TANK
  - OCEAN PROTECT OCEANGUARDS
  - DETENTION TANK
  - SAND FILTER

**STORMWATER CATCHMENT AREA PLAN**

1:100

- DENOTES STORMWATER CATCHMENT AREA BOUNDARY (IMPERVIOUS)
- DENOTES STORMWATER CATCHMENT AREA BOUNDARY (PERVIOUS)
- DENOTES EXISTING SURVEY CONTOUR
- DENOTES NEW SURFACE LEVEL CONTOUR

TOTAL SITE AREA	= 1,637 m <sup>2</sup>
ROOF AREA	= 594 m <sup>2</sup>
PAVEMENT AREA	= 850 m <sup>2</sup>
LANDSCAPE AREA	= 193 m <sup>2</sup>

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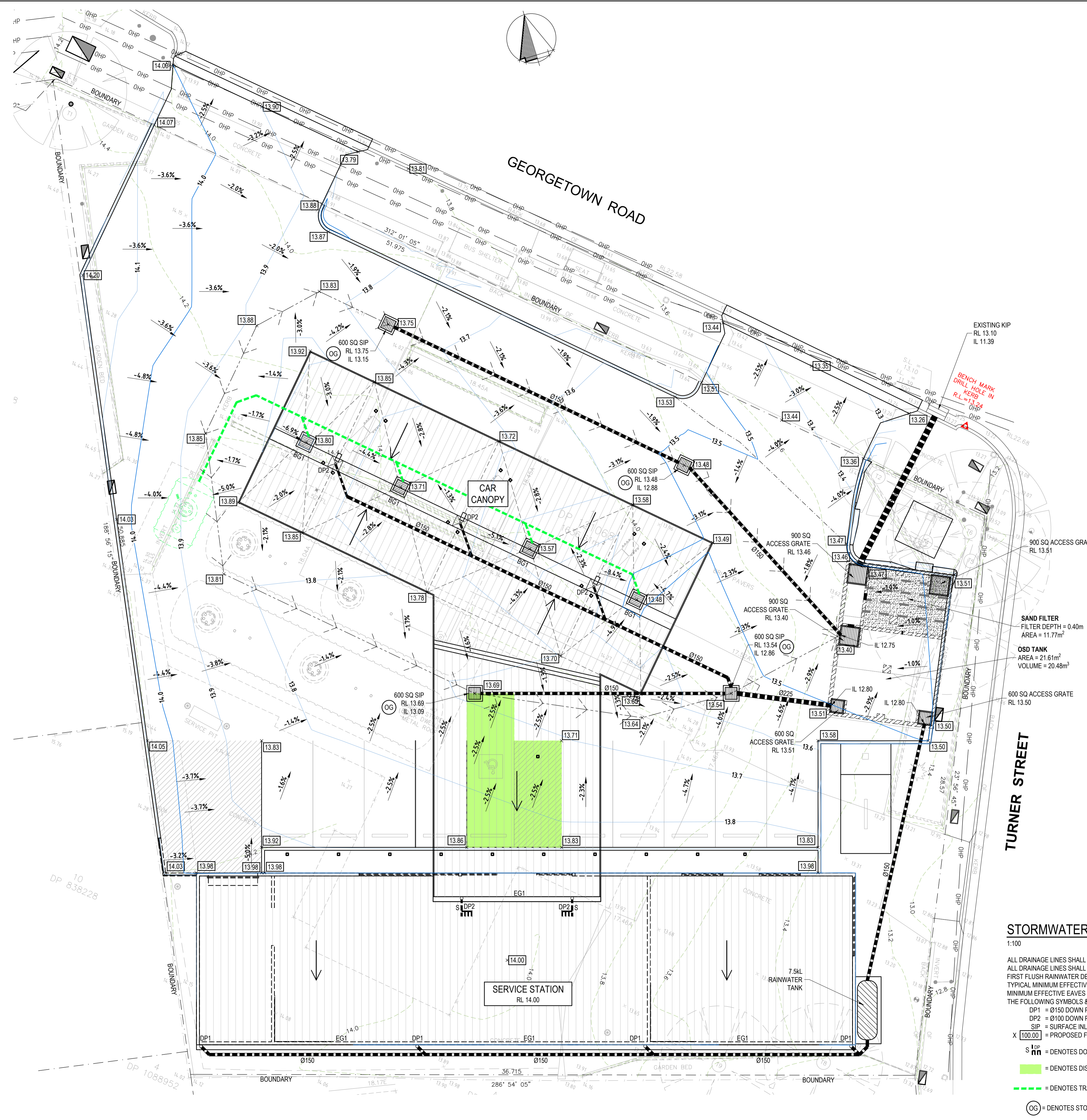
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**PROPOSED DEVELOPMENT**  
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**STORMWATER CATCHMENT AREA PLAN**

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		1:100	C03 - B

AT ORIGINAL SIZE



**STORMWATER DRAINAGE STRATEGY**

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- BOX GUTTERS & DOWNPIPES ARE DESIGNED TO ACCEPT A 1:100 YEAR ARI STORM EVENT.
- ALL PITS & PIPES ARE DESIGNED TO ACCEPT A 1:20 YEAR ARI STORM EVENT.
- DESIGN RAINFALL INTENSITIES:  
1:20 YEAR, 5 MIN = 221 mm/hr  
1:100 YEAR, 5 MIN = 308 mm/hr
- ALL PIPES MUST HAVE A MIN. 1.0% FALL, UNO.
- THE FOLLOWING SQIDs HAVE BEEN DESIGNED FOR THIS DEVELOPMENT
  - RAINWATER TANK
  - OCEAN PROTECT OCEANGUARDS
  - DETENTION TANK
  - SAND FILTER

**STORMWATER CONTROLS**

STORAGE REQUIREMENTS IN ACCORDANCE WITH NCC DCP 7.06.02:

SITE AREA	= 1,637 m <sup>2</sup>
IMPERVIOUS AREA	= 1,444 m <sup>2</sup>
STORAGE REQUIREMENT	= 16.97 mm
STORAGE VOLUME	= 27.78 m <sup>3</sup>
ROOF AREA	= 390 m <sup>2</sup>
MINIMUM RAINWATER TANK VOLUME	= 7.50 m <sup>3</sup>
MINIMUM DETENTION TANK VOLUME	= 20.27 m <sup>3</sup>

ON SITE RETENTION (WITH SAND FILTER) IN ACCORDANCE WITH NCC STORMWATER AND EFFICIENCY FOR DEVELOPMENT 4.3:

CONTRIBUTION CATCHMENT AREA	= 1444 m <sup>2</sup>
MINIMUM SAND FILTER AREA	= 11.55 m <sup>2</sup>
SAND FILTER DEPTH	= 400 mm

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**STORMWATER DRAINAGE PLAN**

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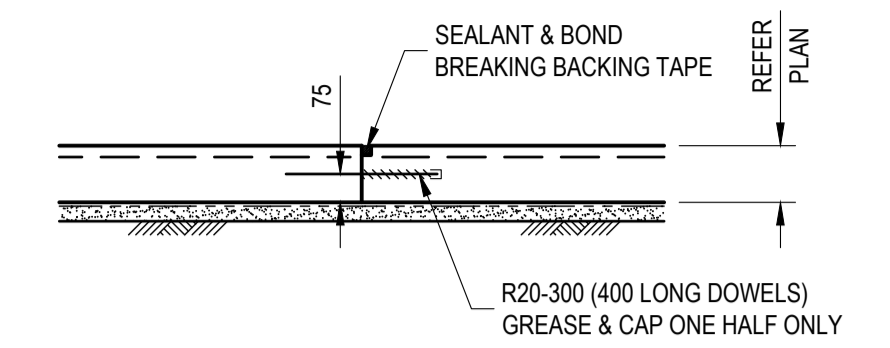
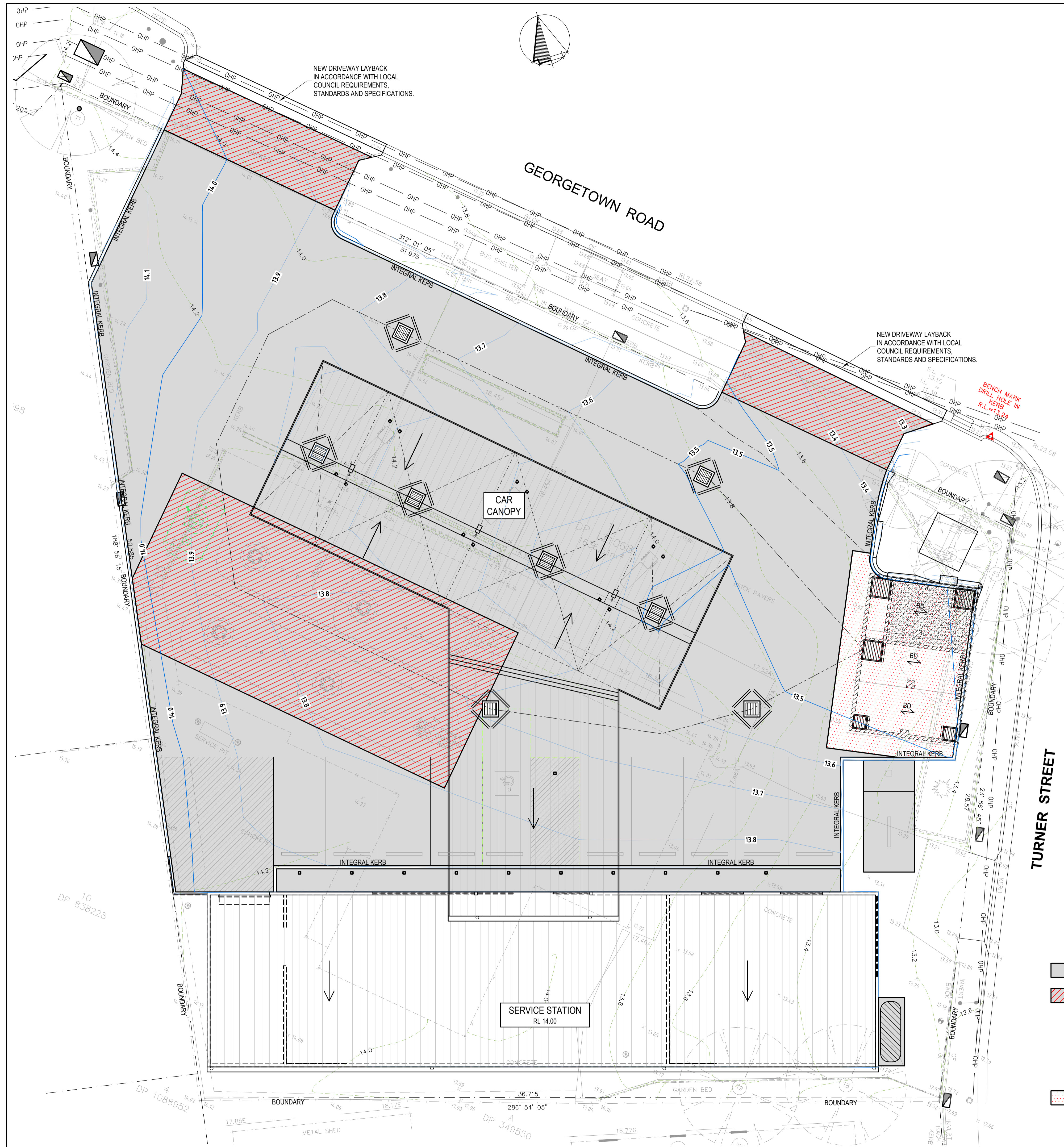
**STORMWATER DRAINAGE PLAN**

1:100

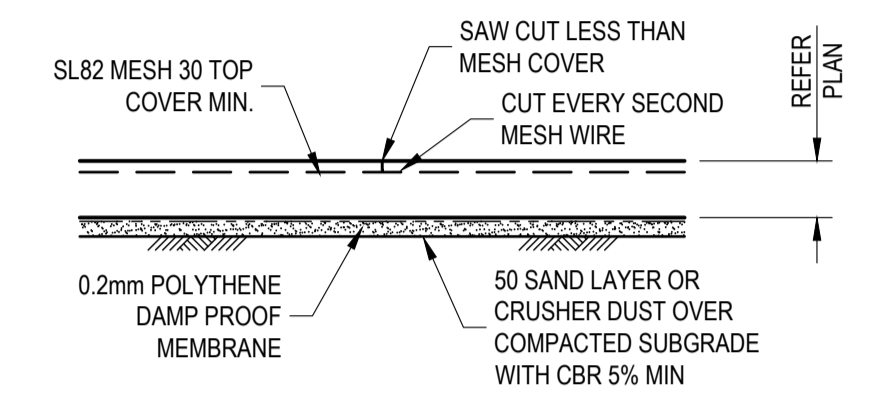
ALL DRAINAGE LINES SHALL BE UPVC (CLASS SH) STORMWATER DRAINAGE PIPE, UNO.  
ALL DRAINAGE LINES SHALL BE LAID @ 1% FALL MIN. UNO.  
FIRST FLUSH RAINWATER DEVICES TO BE FITTED TO DRAINAGE LINES TO BUILDER'S DETAIL.  
TYPICAL MINIMUM EFFECTIVE EAVES GUTTER SIZE = 24,500 mm<sup>2</sup>  
MINIMUM EFFECTIVE EAVES GUTTER SLOPE = 1:500  
THE FOLLOWING SYMBOLS & ABBREVIATIONS HAVE BEEN USED:

- DP1 = Ø150 DOWN PIPE
- DP2 = Ø100 DOWN PIPE
- SIP = SURFACE INLET PIT (NO LINTEL)
- X [100.00] = PROPOSED FINISHED SURFACE LEVEL
- S<sup>top</sup> = DENOTES DOWN PIPE SPREADER
- nn = DENOTES DISABLED PARKING BAY AT 2.5% MAX. GRADE
- = DENOTES TRADE WASTE PIPE
- OG = DENOTES STORMWATER PIT TO BE FITTED WITH OCEAN GUARD - REFER TYPICAL DETAIL

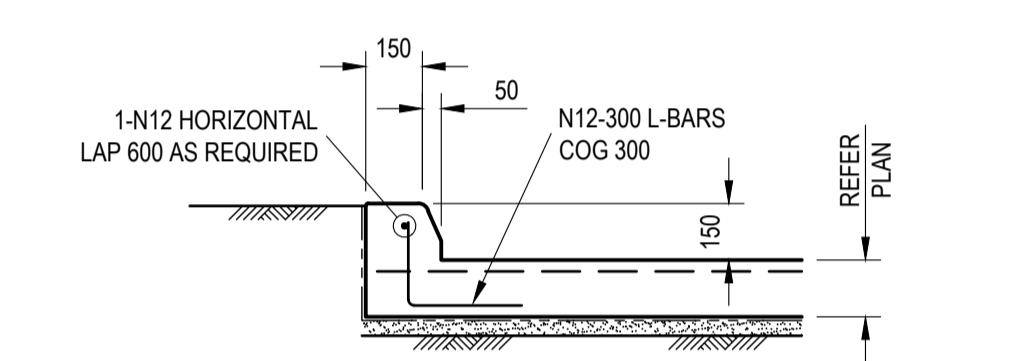
AT ORIGINAL SIZE



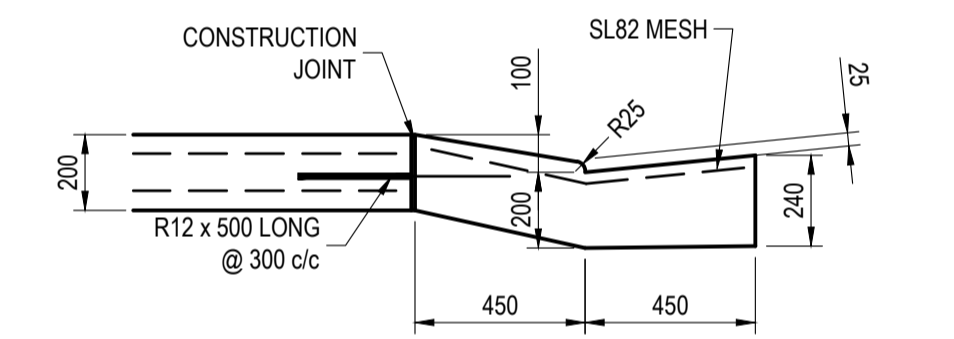
**CONSTRUCTION JOINT (C.J.) DETAIL**  
1:20



**SAWN JOINT (S.J.) DETAIL**  
1:20



**INTEGRAL KERB DETAIL**  
1:20



**TYPICAL HEAVY DUTY LAYBACK DETAIL**  
1:20

**EXTERNAL PAVEMENT LAYOUT PLAN**  
1:100

- DENOTES 150 THICK SLAB WITH SL82 MESH TOP THROUGHOUT CONCRETE STRENGTH = 32 MPa
- DENOTES 200 THICK SLAB WITH SL92 MESH TOP & BOTTOM THROUGHOUT CONCRETE STRENGTH = 32 MPa
- 2-N12 (75 SPACING 1200 LONG) TRIMMERS TOP SHALL BE LOCATED 50 FROM ALL RE-ENTRANT CORNERS, TYPICAL U.N.O.
- REINFORCEMENT COVER TO GROUND FLOOR SLAB SHALL BE AS FOLLOWS:  
40mm - TO UNPROTECTED GROUND  
40mm - EXTERNAL EXPOSURE  
30mm - TO A MEMBRANE IN CONTACT WITH GROUND  
30mm - INTERNAL EXPOSURE
- DENOTES 1.00 BMT BONDEK SLAB SPAN DIRECTION (BY LYSAGHTS OR APPROVED EQUIVALENT) TANK SLAB TO BE 200 THICK WITH SL81 MESH TOP THROUGHOUT & N16-200 BTM IN EACH SPAN WITH N12-300 CROSS RODS BTM 32 MPa MINIMUM CONCRETE STRENGTH, TYPICAL UNLESS NOTED OTHERWISE.

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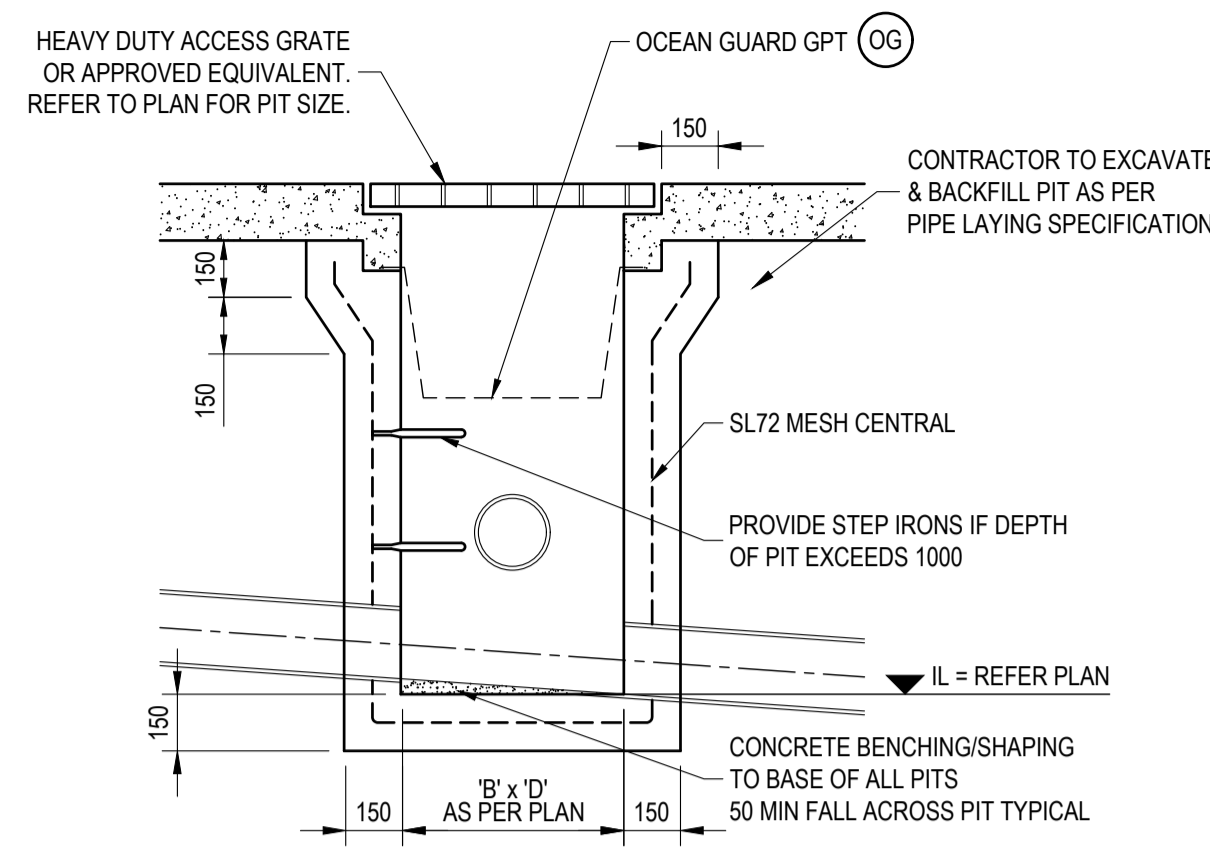
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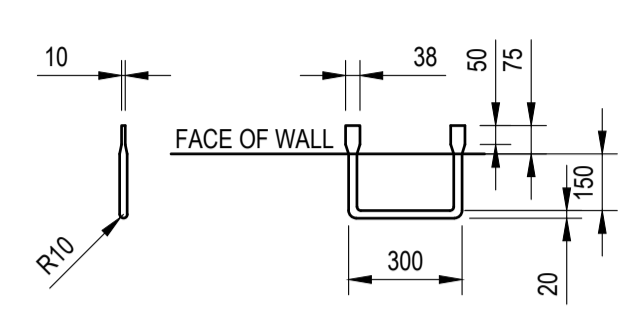
**EXTERNAL PAVEMENT SLAB PLAN & DETAILS**

DESIGN	DRAWN	DATE	PROJECT No.
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		1:100	C05 - B

AT ORIGINAL SIZE

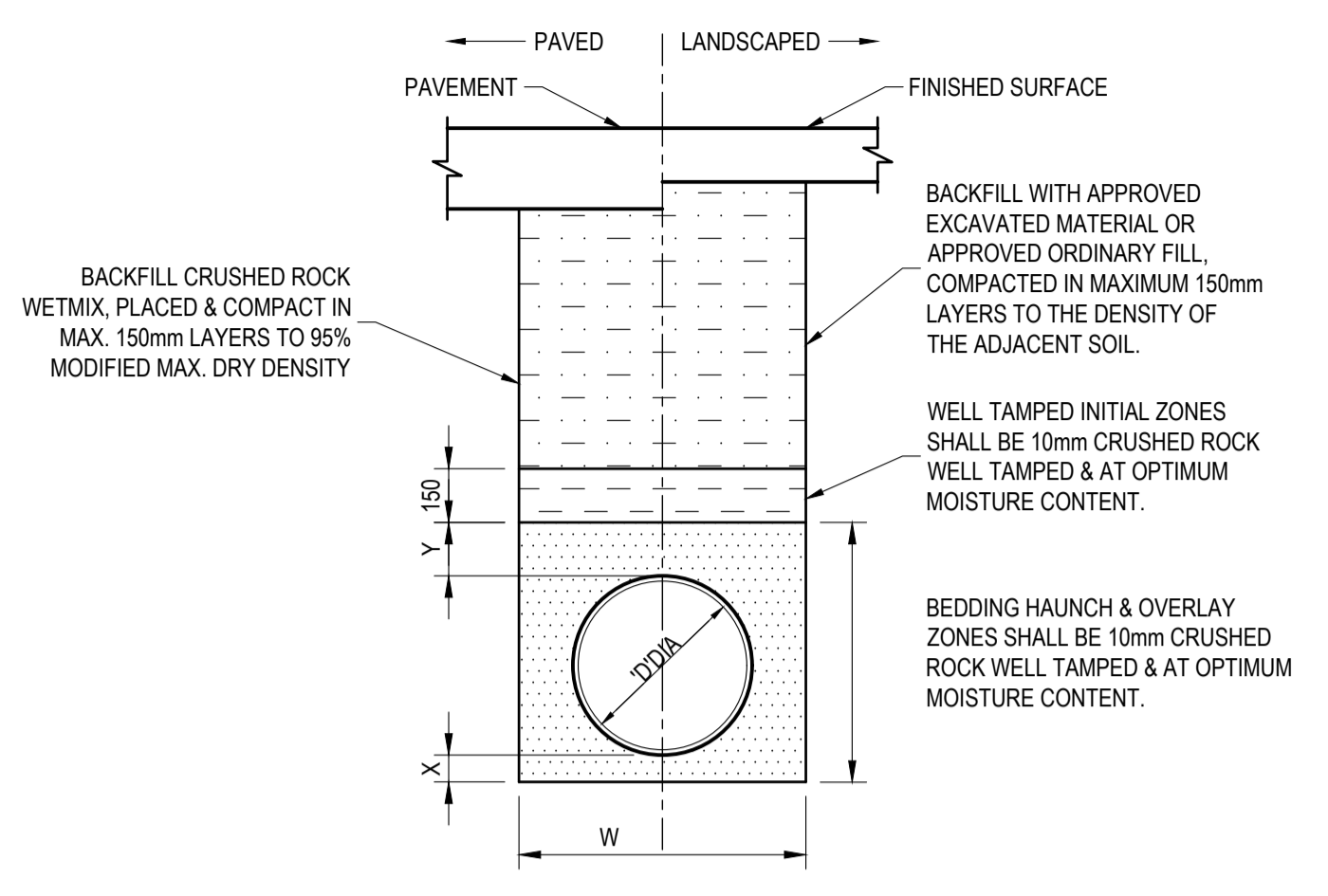


**TYPICAL SURFACE INLET PIT DETAIL**  
1:20  
TYPICAL FOR ALL PITS IN DRIVEWAY/CARPAK AREAS.



**STEP IRONS FOR DRAINAGE PITS**  
NOTE:  
1. FIRST RUNG 150mm DOWN FROM TOP, THEN SPACED AT 300 CENTRES.  
2. STEP IRON MATERIAL, 20m DIAMETER MILD STEEL, HEAVY GALVANISED.

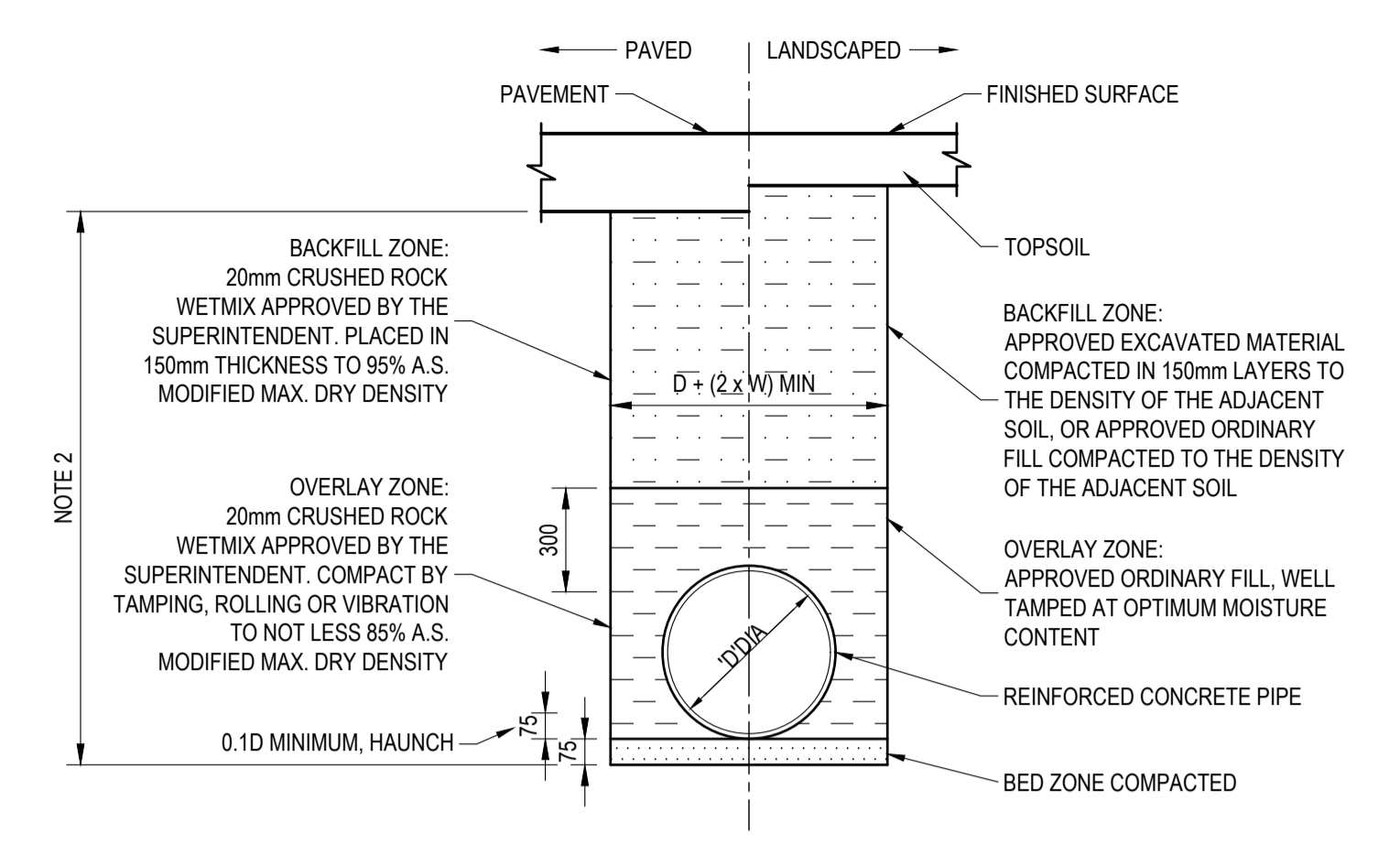
**STEP IRONS FOR DRAINAGE PITS**  
1:20



NOTE:  
1 REFER TO PIPE LAYING SPECIFICATIONS FOR DETAILS.

PIPE DIA 'D'	W	X MIN	Y
100-150	300	75	75
225-300	600	75	75

**UPVC PIPE**

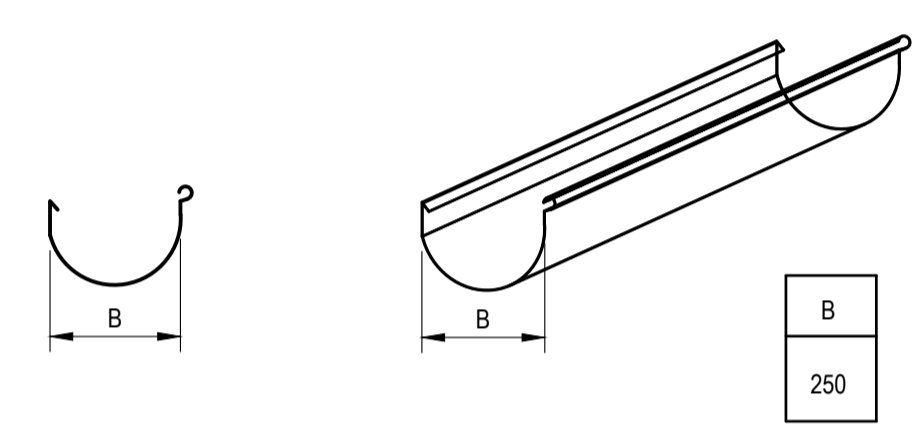


NOTE:  
1 REFER TO PIPE LAYING SPECIFICATION FOR DETAILS.  
2 BACKFILL OVERLAY & BEDDING ZONES 20mm CRUSHED ROCK COMPACT BY TAMPING ROLLING OR VIBRATION TO NOT LESS THAN 85% A.S. STD. MAX. DRY DENSITY.

D	W
150-300	150
375-750	300
+750	600

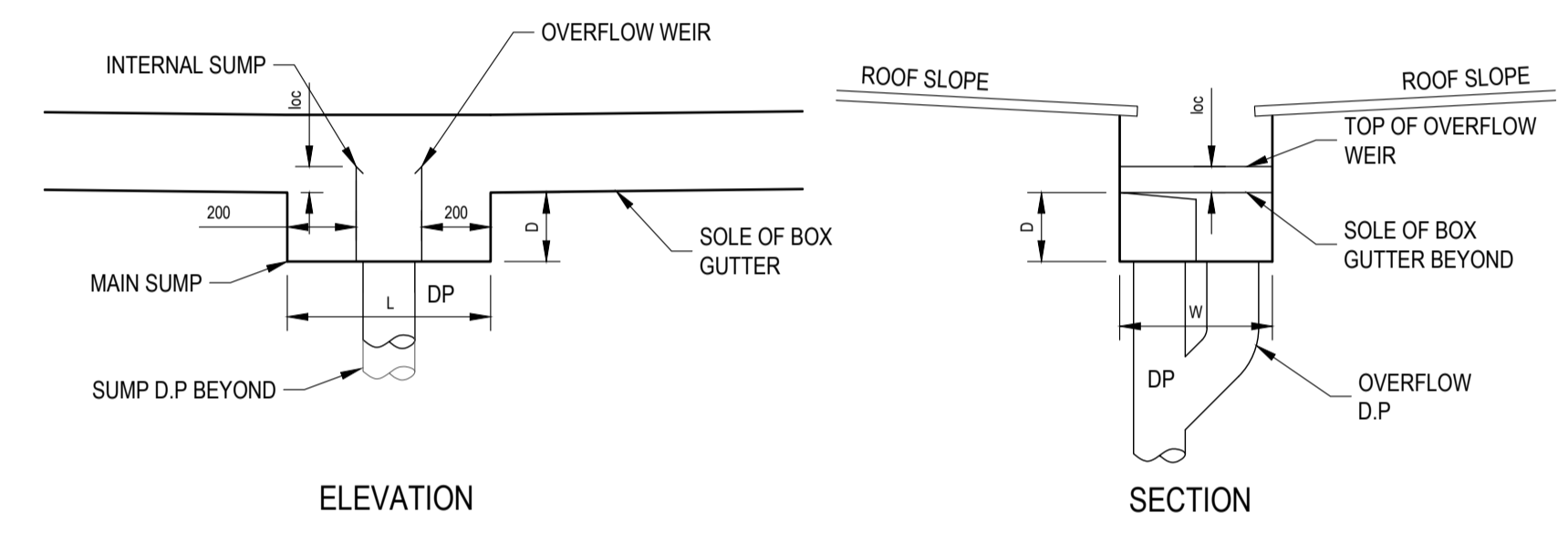
**REINFORCED CONCRETE PIPE**

**TYPICAL PIPE LAYING DETAIL**  
1:20

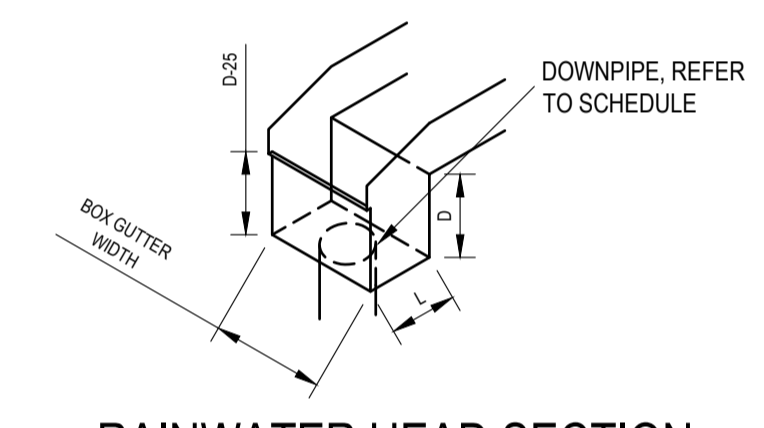


**TYPICAL EAVES GUTTER DETAIL**  
NTS

MARK	GUTTER TYPE	'B'	TOTAL CROSS SECTION AREA (mm <sup>2</sup> )	DOWNPIPE SIZE
EG1	250 HALF ROUND	250	24500	Ø150

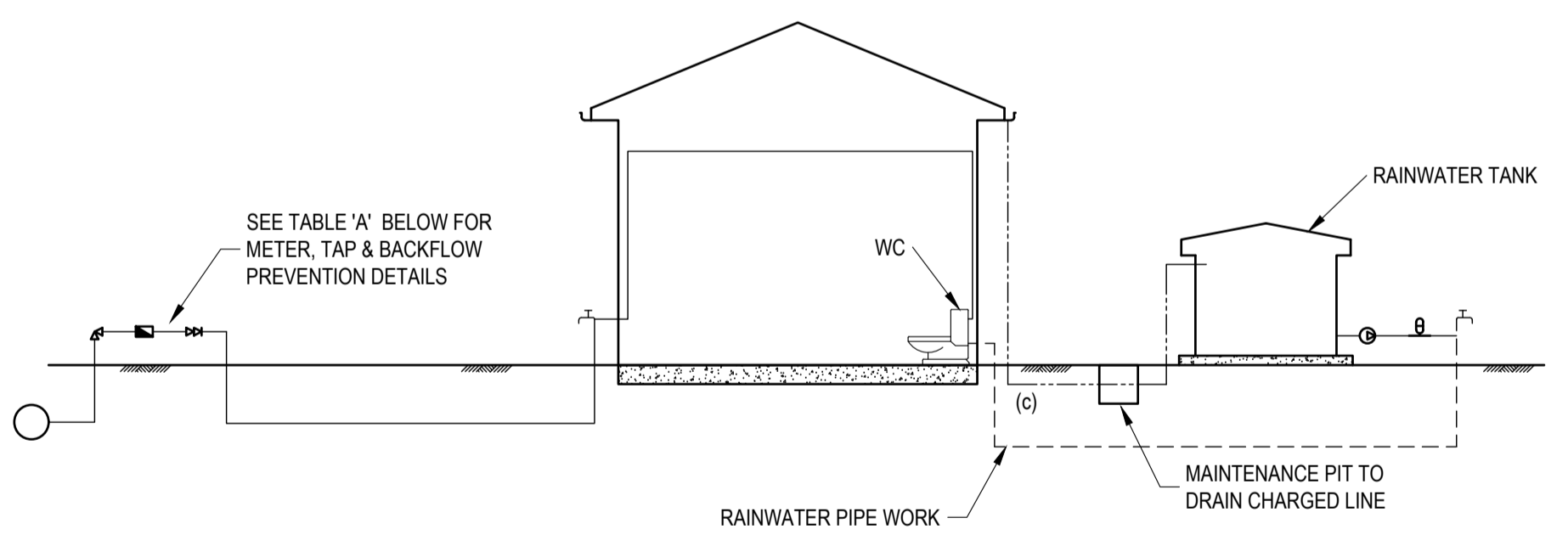


**TYPICAL BOX GUTTER DETAIL WITH VERTICAL SUMP OVERFLOW**  
1:20



**RAINWATER HEAD SECTION**  
NTS

NODE	BOX GUTTER SIZE	RAINWATER HEAD SIZE	SUMP SIZE	loc (mm)	DOWNPIPE Ø mm
BG1	600 W x 150 D	N/A	600 L x 200 D	20mm	150



**LEGEND**

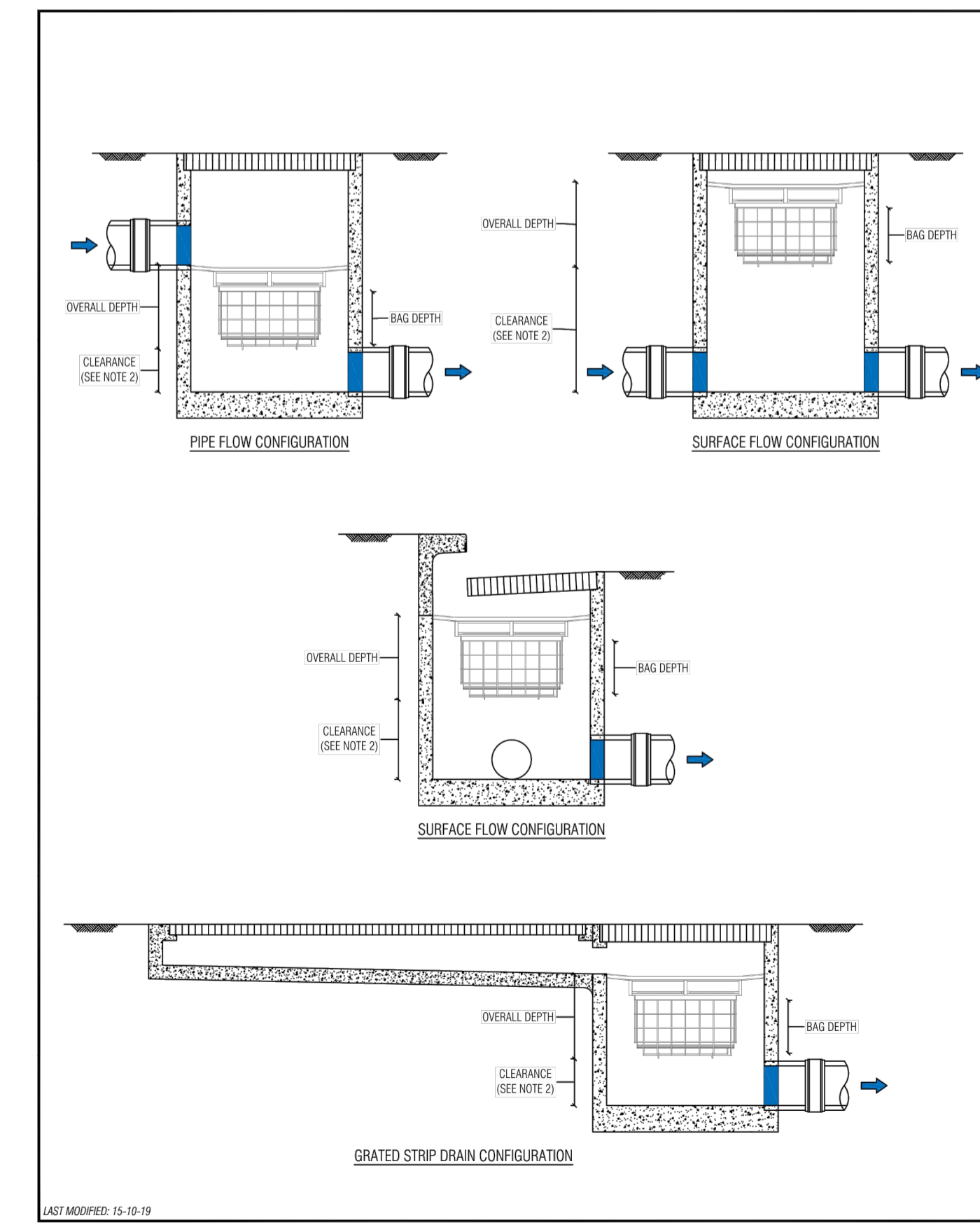
- ⊠ PRESSURE VESSEL
- ⊠ METER
- ⊠ BALL VALVE RIGHT ANGLE TYPE
- ⊠ DUAL CHECK VALVE
- ⊠ PUMP
- ⊠ GARDEN TAP
- DRINKING WATER SUPPLY PIPES
- RAINWATER SUPPLY PIPES
- DOWN PIPES

**TABLE A**

RAINWATER TANK LOCATION	METER SIZE (mm)	TYPE OF TAP	TYPE OF BACKFLOW PREVENTION
ABOVE GROUND	20	BALL VALVE	DUAL CHECK VALVE (COMBINED WITH METER)
	25	BALL VALVE	DUAL CHECK VALVE
	≥ 32	BALL VALVE	DUAL CHECK VALVE
BELOW GROUND	20	BALL VALVE	TESTABLE DOUBLE CHECK VALVE
	25	BALL VALVE	TESTABLE DOUBLE CHECK VALVE
	≥ 32	BALL VALVE	TESTABLE DOUBLE CHECK VALVE

**DIAGRAM NOTES:**  
DRAWING TO BE READ IN CONJUNCTION WITH SYDNEY WATER PLUMBING REQUIREMENTS  
1. FOR TANKS 10,000 LITRES OR LESS, COUNCIL DEVELOPMENT CONSENT IS NOT REQUIRED, IF THEIR CONDITIONS FOR INSTALLATION ARE FOLLOWED.  
2. FOR TANKS GREATER THAN 10,000 LITRES COUNCIL DEVELOPMENT CONSENT IS GENERALLY REQUIRED.  
3. FOR TANKS MORE THAN 10,000 LITRES APPROVAL IS REQUIRED FOR BUILDING OVER SEWERS.  
4. SYDNEY WATER'S APPROVAL IS REQUIRED FOR ANY TOP UP FROM DRINKING WATER SUPPLY, REGARDLESS OF TANK SIZE.  
5. NO DIRECT CONNECTION IS ALLOWED BETWEEN THE DRINKING WATER SUPPLY AND THE RAINWATER TANK SUPPLY.  
6. RAINWATER PIPEWORK IS SHOWN ON THE DIAGRAM AS SUPPLYING INTERNAL RAINWATER USES.  
7. ANY DESIGNED ACCESS LID INTO RAINWATER RE-USE TANK IS TO HAVE A LOCKABLE LID. IF THE LID IS DESIGNED TO BE ACCESSED BY A MAINTENANCE PERSON, IT MUST BE AT LEAST 600 mm x 900 mm IN SIZE.  
8. MAINS WATER TO BYPASS TO TANK (BY PLUMBER) FOR LOW TANK STORAGE.

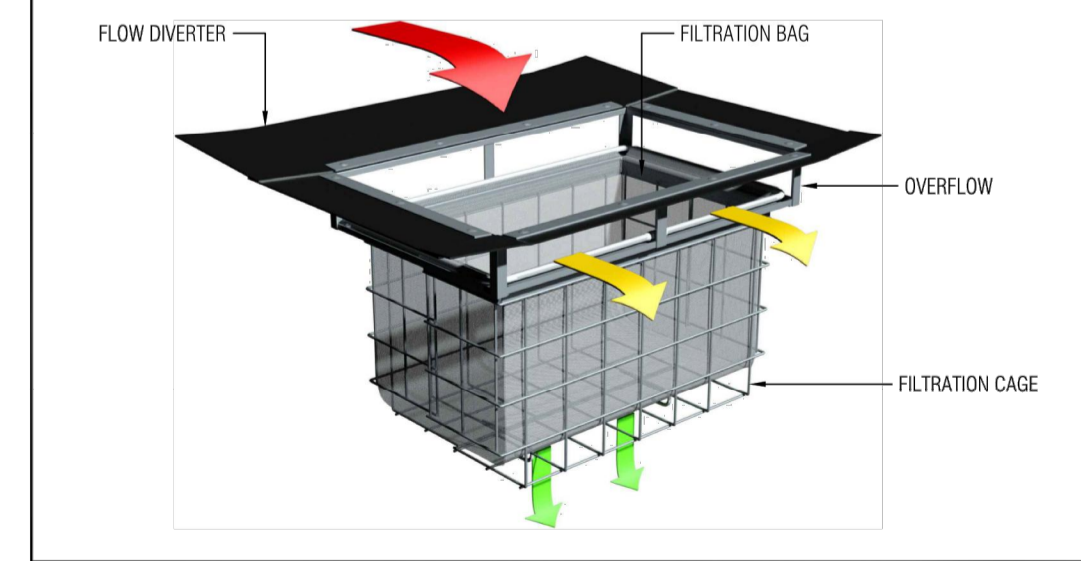
**DUAL WATER & RAINWATER SUPPLY DIAGRAM**  
N.T.S.  
THE RAINWATER TANK SHALL BE INSTALLED WITH A FIRST FLUSH DEVICE TO SUPPLIERS DETAILS  
MAINTENANCE PIT TO BE CONSTRUCTED TO DRAIN CHARGED LINE AS REQUIRED



PLAN ID	MAXIMUM PIT PLAN DIMENSIONS
S	450mm x 450mm
M	600mm x 600mm
L	900mm x 900mm
XL	1200mm x 1200mm

DEPTH ID	BAG DEPTH	OVERALL DEPTH
1	170	270
2	300	450
3	600	700

PLAN ID	DEPTH ID		
	1	2	3
S	•	•	•
M	•	•	•
L	•	•	•
XL	•	•	•



**GENERAL NOTES**  
1. THE MINIMUM CLEARANCE DEPENDS ON THE CONFIGURATION (SEE NOTE 2) AND THE LOCAL COUNCIL REQUIREMENTS.  
2. CLEARANCE FOR ANY PIT (WITHOUT AN INLET PIPE (ONLY USED FOR SURFACE FLOW)) CAN BE AS LOW AS 50mm. FOR OTHER PITS, THE RECOMMENDED CLEARANCE SHOULD BE GREATER OR EQUAL TO THE PIPE OBVERT SO AS NOT TO INHIBIT HYDRAULIC CAPACITY.  
3. OCEAN PROTECT PROVIDES TWO FILTRATION BAG TYPES- 200 MICRON BAGS FOR HIGHER WATER QUALITY FILTERING AND A COARSE BAG FOR TARGETING GROSS POLLUTANTS.  
4. DRAWINGS NOT TO SCALE.



OCEAN PROTECT  
OCEANGUARD  
TYPICAL ARRANGEMENTS  
SPECIFICATION DRAWING

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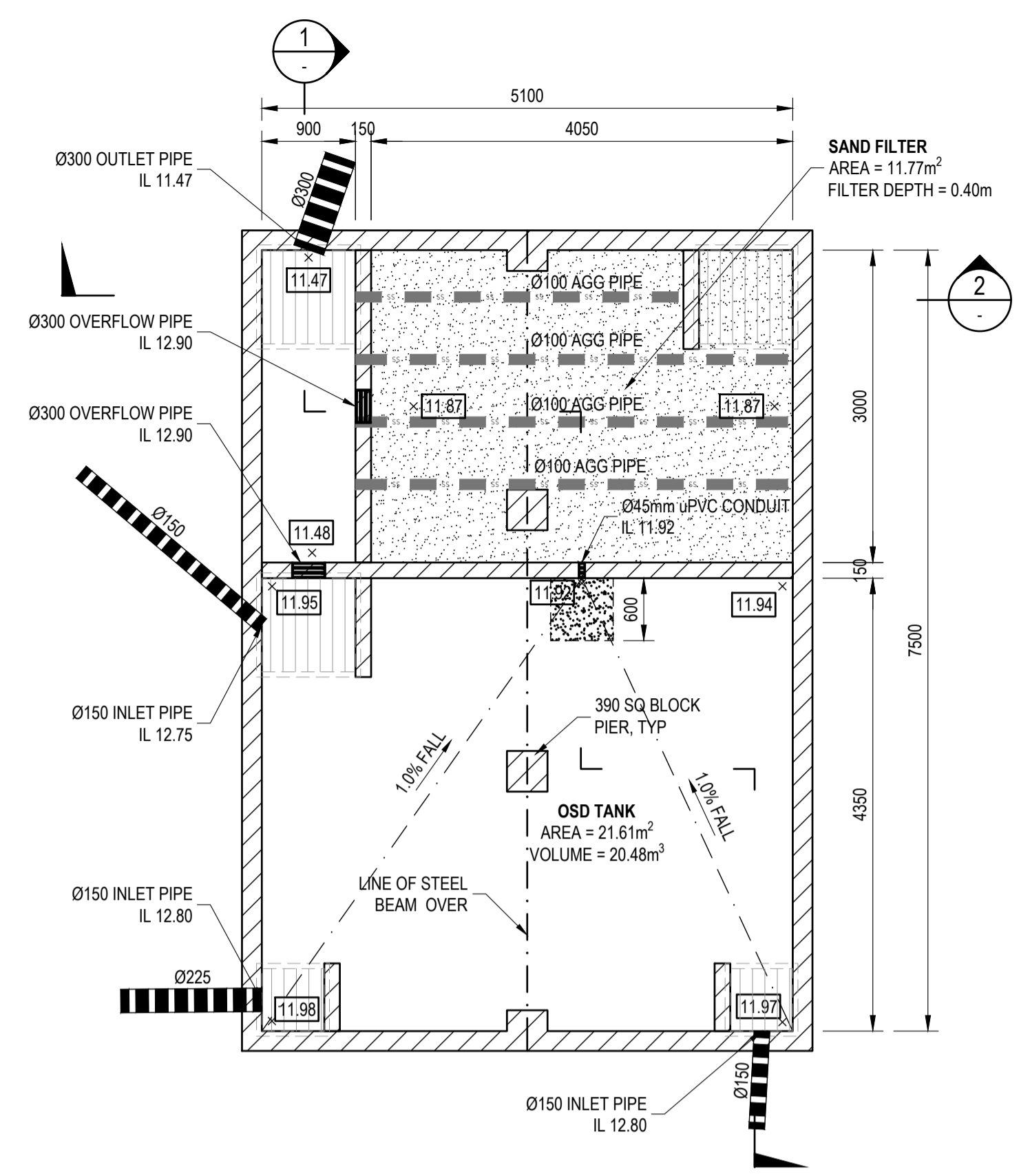
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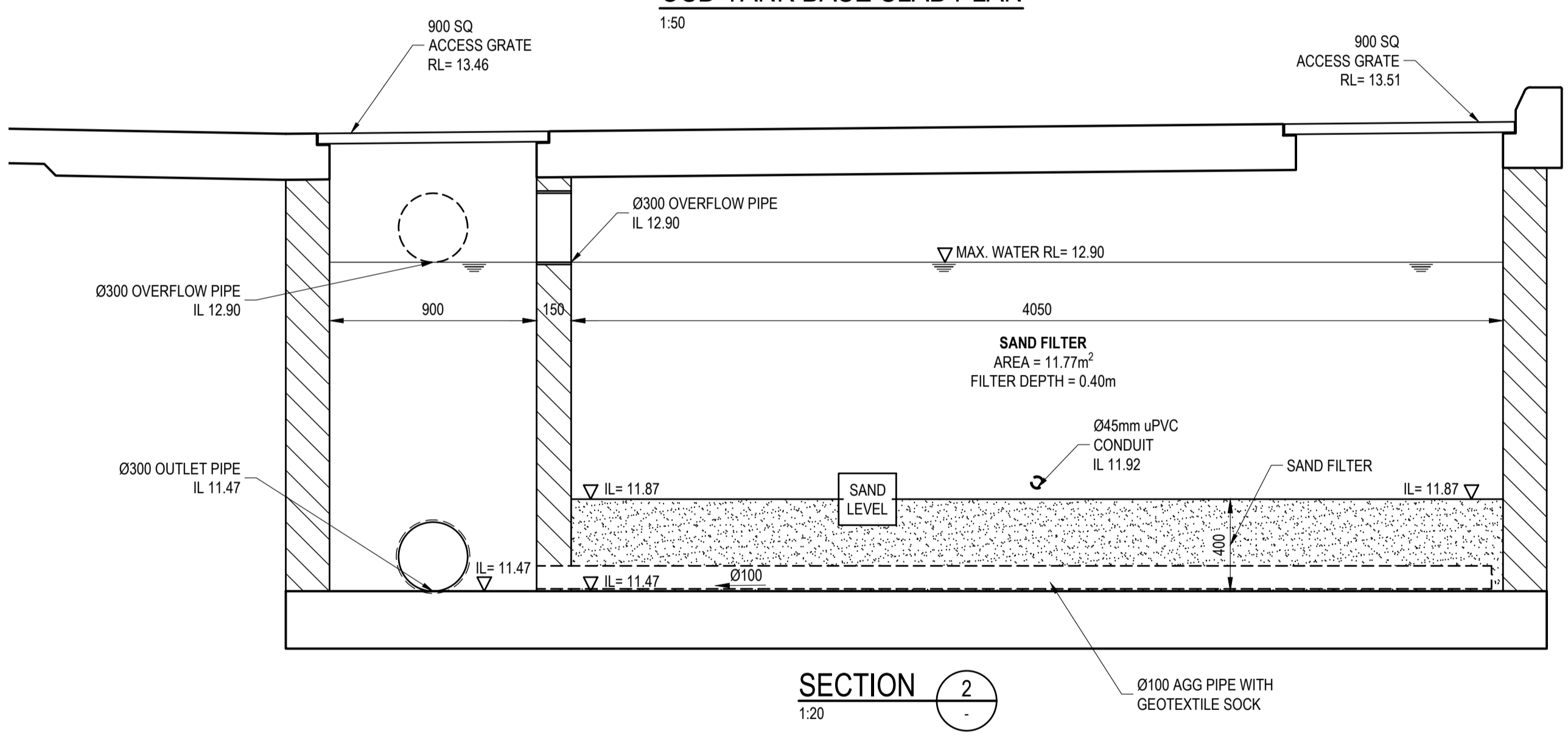
**PROPOSED DEVELOPMENT**  
42 Georgetown Road, Georgetown  
For GWH Build Pty Ltd

**STORMWATER DETAILS SHEET 1 OF 2**

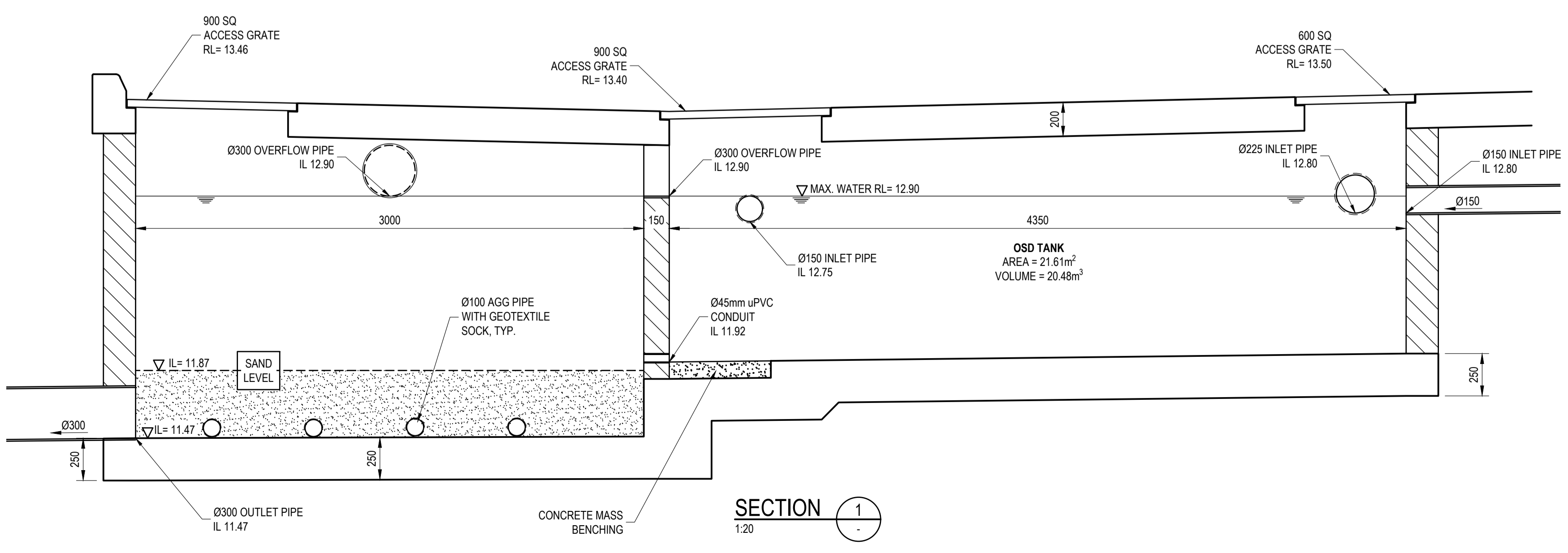
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		1:20, 1:50	C06 - B



OSD TANK BASE SLAB PLAN  
1:50



SECTION 2  
1:20



SECTION 1  
1:20

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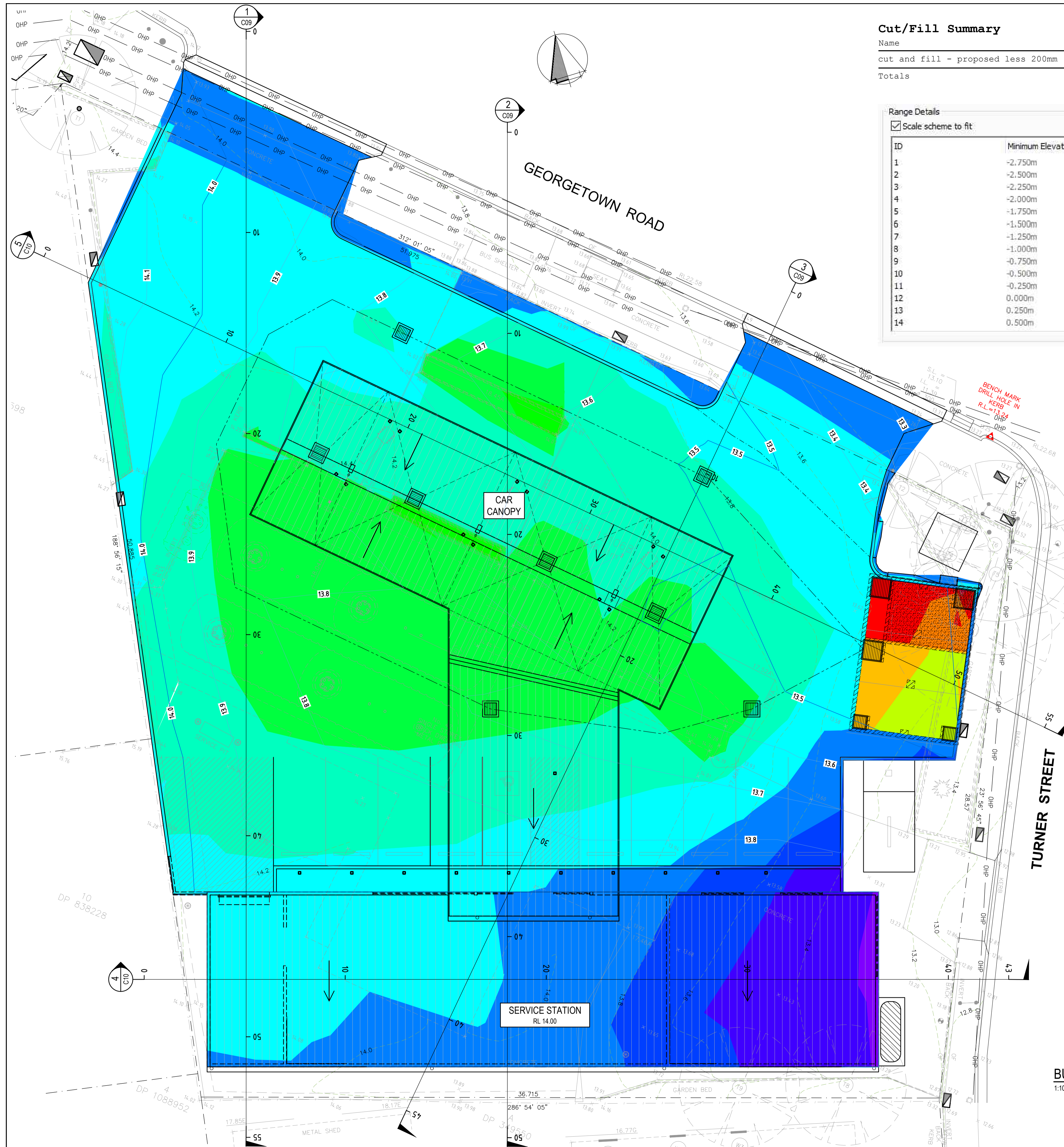
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AT ORIGINAL SIZE





**Cut/Fill Summary**

Name	Cut Factor	Fill Factor	2d Area	Cut	Fill	Net
cut and fill - proposed less 200mm	1.000	1.000	1500.897sq.m	765.557 Cu. M.	32.660 Cu. M.	732.897 Cu. M.<Cut>
Totals			1500.897sq.m	765.557 Cu. M.	32.660 Cu. M.	732.897 Cu. M.<Cut>

**Range Details**

Scale scheme to fit

ID	Minimum Elevation	Maximum Elevation	Color Scheme
1	-2.750m	-2.500m	
2	-2.500m	-2.250m	
3	-2.250m	-2.000m	
4	-2.000m	-1.750m	
5	-1.750m	-1.500m	
6	-1.500m	-1.250m	
7	-1.250m	-1.000m	
8	-1.000m	-0.750m	
9	-0.750m	-0.500m	
10	-0.500m	-0.250m	
11	-0.250m	0.000m	
12	0.000m	0.250m	
13	0.250m	0.500m	
14	0.500m	0.750m	

CUT  
FILL

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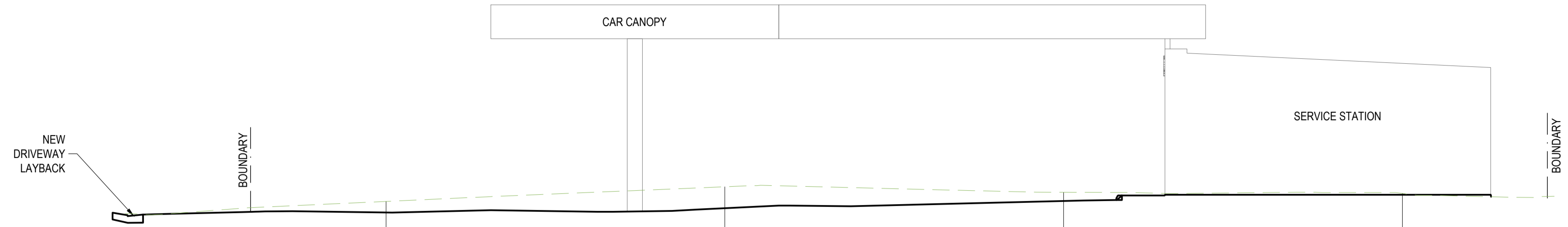
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**BULK EARTHWORKS CUT AND FILL PLAN**

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**BULK EARTHWORKS CUT AND FILL PLAN**  
1:100

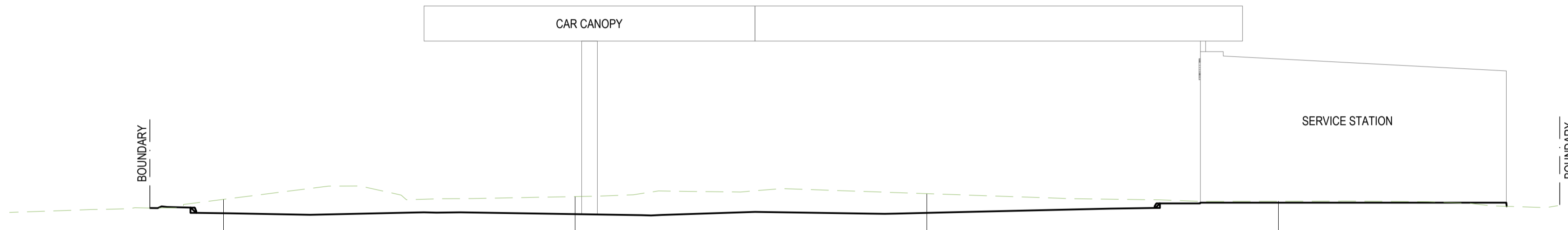
AT ORIGINAL SIZE



VERT EXAG 1:1  
Datum 10.000

DESIGN LEVELS																		
EXISTING LEVELS																		
DEPTH																		
CHAINAGE	0.000	2.826	6.001	10.000	10.175	13.093	20.000	21.593	23.717	30.000	32.992	40.000	42.607	42.618	44.279	45.000		

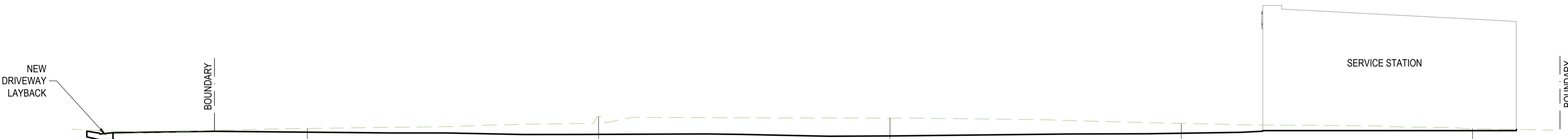
SITE CROSS SECTION 3  
1:100



VERT EXAG 1:1  
Datum 10.000

DESIGN LEVELS																		
EXISTING LEVELS																		
DEPTH																		
CHAINAGE	0.000	7.911	10.000	12.464	15.703	20.000	25.116	28.802	30.000	37.781	40.000	46.481	46.491	48.000	50.000	50.000		

SITE CROSS SECTION 2  
1:100



VERT EXAG 1:1  
Datum 10.000

DESIGN LEVELS																		
EXISTING LEVELS																		
DEPTH																		
CHAINAGE	0.000	3.330	6.817	10.000	17.405	20.000	27.913	30.000	40.000	42.797	50.000	51.497	51.507	53.000	55.000			

SITE CROSS SECTION 1  
1:100

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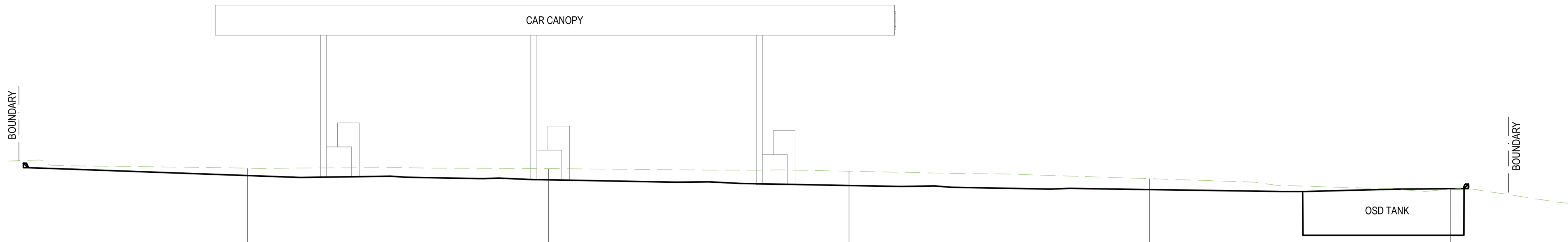
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SITE CROSS SECTIONS SHEET 1 OF 2

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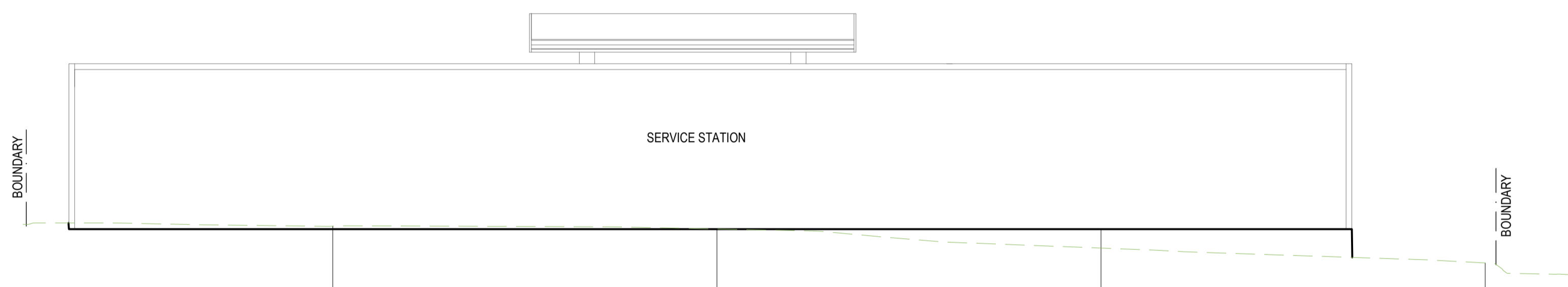
AT ORIGINAL SIZE



VERT EXAG 1:1  
Datum 10.000

DESIGN LEVELS		14.429	14.343		13.937	13.874	13.916	13.795	13.605	13.512	13.471	13.423	13.500	13.504	13.553	
EXISTING LEVELS		14.429	14.433		14.179	14.186	14.201	14.165	14.079	13.918	13.831	13.561	13.481	13.500	13.492	13.303
DEPTH		-0.090			-0.242	-0.313	-0.285	-0.370	-0.473	-0.407	-0.361	-0.137	0.018	0.004	0.061	
CHAINAGE	0.000	2.387	2.530		10.000	11.743	14.743	20.000	30.000	37.343	40.000	45.834	50.000	50.450	50.616	51.930

SITE CROSS SECTION 5  
1:100



VERT EXAG 1:1  
Datum 10.000

DESIGN LEVELS		14.124	14.161	14.161	14.000	14.000	14.000	14.000	13.509	14.000	14.000	13.267	13.267	13.122	13.085
EXISTING LEVELS		14.124	14.161	14.161	14.077	14.005	14.005	14.005	13.509	13.267	13.267	13.122	13.085		
DEPTH		0.000	-0.161	-0.161	-0.077	-0.005	-0.005	-0.005	0.491	0.733	0.000	0.000	0.000		
CHAINAGE	0.000	2.019	3.120	3.130	10.000	20.000	30.000	36.530	36.540	40.000	40.282	43.000			

SITE CROSS SECTION 4  
1:100

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AT ORIGINAL SIZE



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**PV 19/10/21 – 42 Georgetown Road Georgetown - DA2021/00197 -  
Service station and ancillary food and drink premises including  
demolition of existing structures**

**ITEM-2            Attachment B:      Processing Chronology**

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**PROCESSING CHRONOLOGY**

**DA2021/00187 – 42 Georgetown Road, Georgetown**

19 February 2021	Application lodged
11 March 2021 – 25 March 2021	Public notification - five submissions received
15 March 2021	Additional information regarding contamination/remediation and odour.
17 March 2021	Additional information from the Applicant received.
13 May 2021	Additional information requested regarding remediation action plan detail.
18 May 2021	Additional information from the Applicant received.
4 August - 1 September 2021	Second round of public exhibition - eleven submissions received
27 August 2021	Additional information requested regarding remediation action plan detail.
02 September 2021	Additional information from the Applicant received.